



**2015 Monitoring Report  
Jones Road Landfill Site**

Part of Lots 1 and 2 Concession I,  
and Part of Lots 1 and 2, Concession II,  
Townships of Pettypiece and Jackman,  
District of Kenora

Prepared for:  
The City of Kenora

Prepared by:  
Azimuth Environmental  
Consulting, Inc.

June 2015

AEC 15-020



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Environmental Assessments & Approvals

June 10, 2015

AEC 15-020

City of Kenora  
Waste Management Department  
60 14<sup>th</sup> Street  
Kenora, Ontario  
P9N 4M9

Attention: Mukesh Pokharel, P.Eng.  
Environmental Supervisor

**Re: 2015 Report on the Monitoring Programs at the Jones Road Landfill Site**

Dear Mr. Pokharel:

Azimuth Environmental Consulting, Inc. (Azimuth) is pleased to present our report on the 2012 - 2014 monitoring program conducted at the Jones Road Landfill Site. In general, the monitoring data obtained over this period indicates that the landfill is not causing unacceptable impact on the surrounding environment. Currently, no quantifiable impacts to the ground water regime are observed within either the landfill footprint or at monitors situated immediately downgradient of the waste cells. In addition, only minor impacts were observed at SW-1 (closest to fill area), which are assumed to be a result of the construction/demolition and sewage sludge waste and the recent addition of municipal waste. Slight water quality impacts were also measured at SW-2 and SW-3 although to a much lesser degree than observed at SW-1.

We would like to thank you for the opportunity to complete this project. If you have any questions or comments, please contact the undersigned.



Yours truly,

AZIMUTH ENVIRONMENTAL CONSULTING, INC.



Colin Ross, B.Sc., P.Geo.  
Hydrogeologist

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Attach:

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## Table of Contents

	page
<b>1.0 INTRODUCTION &amp; BACKGROUND.....</b>	<b>1</b>
<b>1.1 Location .....</b>	<b>1</b>
<b>1.2 Geology.....</b>	<b>1</b>
1.2.1 Peat .....	1
1.2.2 Clayey Silt .....	2
1.2.3 Silty Sand Till.....	2
1.2.4 Bedrock.....	3
1.2.5 Hydrogeology .....	3
1.2.6 Hydrology .....	3
<b>2.0 SITE DESIGN &amp; OPERATIONS .....</b>	<b>4</b>
<b>2.1 Operation Changes .....</b>	<b>5</b>
<b>2.2 Cover Requirements, Erosion Protection.....</b>	<b>5</b>
<b>3.0 SUMMARY OF 2012-2014 MONITORING PROGRAMS .....</b>	<b>5</b>
<b>3.1 Monitoring Program Omissions .....</b>	<b>7</b>
3.1.1 Ground Water .....	7
3.1.2 Surface Water .....	7
<b>3.2 Quality Assurance and Quality Control Results.....</b>	<b>8</b>
<b>3.3 Ground Water &amp; Leachate (Schedule “F”).....</b>	<b>8</b>
3.3.1 Ground Water & Leachate Flow .....	9
3.3.2 Background Ground Water Quality.....	10
3.3.3 Leachate Quality.....	10
3.3.4 Downgradient Ground Water Quality .....	11
<b>3.4 Surface Water (Schedule “C”).....</b>	<b>11</b>
3.4.1 Surface Water Flow .....	12
3.4.2 Surface Water Quality .....	13
<b>3.5 Trigger Mechanism Assessment .....</b>	<b>14</b>
<b>3.6 Sediment (Schedule “D”).....</b>	<b>15</b>
<b>3.7 Biological Effects (Schedule “E”) .....</b>	<b>16</b>
<b>3.8 Landfill Gas (Schedule “G”).....</b>	<b>16</b>
<b>3.9 Geotechnical Monitoring (Schedule “H”).....</b>	<b>16</b>
<b>4.0 PROPOSED 2015 -2017 MONITORING PROGRAM .....</b>	<b>17</b>
<b>5.0 CONCLUSIONS AND RECOMMENDATIONS .....</b>	<b>18</b>
<b>6.0 REFERENCES.....</b>	<b>18</b>



### **List of In-Text Tables**

Table 1: Summary of the Required 2012-2014 Monitoring Programs .....	6
Table 2: Summary of Notable QA/QC Variations.....	8
Table 3: Background Chemistry .....	10
Table 4: Surface Water Flow Data.....	12
Table 5: Proposed Trigger Limits – MW3/8.....	14
Table 6: Proposed Surface Water Trigger Limits .....	15
Table 7: Summary of the Required 2015-2017 Monitoring Programs .....	17

### **List of Figures**

Figure 1	Site Location
Figure 2	Hydrologic Features
Figure 3	Landfill Development Sequence
Figure 4	Ground Water Monitoring Locations
Figure 5	Ground Water Monitoring (May 2014)
Figure 6	Saddle Ground Water Elevations (overburden)
Figure 7	East-West Cross Section A-A <sup>1</sup>
Figure 8	South North Cross Section B-B <sup>1</sup>

### **List of Appendices**

Appendix A: Figures
Appendix B: MOE Correspondence and Certificates of Approval
Appendix C: Annual Waste Quantities
Appendix D: Ground Water Quality Data
Appendix E: Ground Water Levels
Appendix F: Ground Water Chemistry Over Time Graphs
Appendix G: Detected Volatile Organic Compounds
Appendix H: Surface Water Quality Data
Appendix I: Surface Water Quality Over Time Graphs
Appendix J: Sediment Sampling Data
Appendix K: Eng-Tech 2014 Report
Appendix L: Final Contour Design Drawing
Appendix M: Borehole Logs
Appendix N: Benthic Invertebrate Monitoring Data
Appendix O: MOE Landfill Reporting Submission Forms



## **1.0 INTRODUCTION & BACKGROUND**

The following information is provided as a summary of previous investigations to allow the reader to review this report in context. For more complete information, the reader should review the original documentation (see Section 5.0 References).

### **1.1 Location**

The Jones Road landfill site is located immediately to the north of Highway 671 (Jones Road), and approximately 22 km northeast of the City of Kenora. Specifically, the site is located upon Part of Lots 1 and 2 of Concession I, and Part of Lots 1 and 2 of Concession II, in the Townships of Pettypiece and Jackman, of the District of Kenora (Figure 1). Construction of the site began on June 1, 2000, and was essentially complete by the end of the calendar year. The site is operated by the City of Kenora Solid Waste Department and is licensed by the Ministry of the Environment (Ministry) to receive “domestic, commercial, non-hazardous solid industrial and institutional, processed organic sewage sludge, sludge from future municipal or provincial fresh water treatment facilities, non-pathological agricultural and bio-medical, and grit and screenings from street cleaning and sediment basin clean-outs” under Certificate of Approval #A612018 (replaced C of A #A612016 (former MNR landfill)).

### **1.2 Geology**

The Jones Road landfill site is situated within an interconnected bog/wetland complex that is surrounded by various bedrock ridges and knobs. In 1996, the local Quaternary and bedrock geology was characterized by Fenco MacLaren Inc. (Fenco) during an extensive field evaluation of the property. During this evaluation, they observed that the overburden within the bog was stratified, with three predominant layers observed, each of which is discussed in more detail below.

#### **1.2.1 Peat**

The surficial material existing within the bog consists of a dark brown to black peat. Field observations of the material indicate that the peat is fibrous at surface, consisting primarily of undecomposed organic material (i.e., rootlets, leaf litter). With increasing depth, the material becomes more hemic (i.e., muckier) as the proportion of decomposed material increases. The thickness of the peat within the bog was assessed through drilling and seismic monitoring by Fenco, and was found to range from essentially zero at the outer boundary to greater than 3 m at the center of the bog.



Testing of the vertical hydraulic conductivity of the peat was also conducted by Fenco during their evaluation. The results of this testing indicated a rate of  $5 \times 10^{-7}$  to  $9 \times 10^{-8}$  m/s under natural site conditions, given a porosity range of 0.81 to 0.95. Simulation of the vertical stress that the waste will potentially have upon the peat was also assessed through consolidation of the material under pressures of up to 800 kPa. In response to compaction, the vertical hydraulic conductivity of the material was observed to decrease by approximately three orders of magnitude to  $5 \times 10^{-11}$  m/s. The application of vertical pressure upon the peat also resulted in a decrease in the porosity of the material to about 0.67.

The horizontal hydraulic conductivity of the peat was only estimated during excavation of the peat, and was inferred to be much greater than vertical, in the order of about  $10^{-5}$  m/s.

### 1.2.2 Clayey Silt

A layer of clayey silt exists beneath the surficial peat. Colouration of this material is variable, and provides an indicator of the degree of weathering the material has undergone. Unweathered material within this layer is generally blue-grey, while the weathered material possessed a green-brown colouration. During drilling by Fenco in 1996, it was determined that the material becomes siltier with depth, and that several thin, discontinuous sand seams were observed within the profile. The thickness of this material was found to range from approximately 2 to 8 m.

As with the overlying peat, vertical hydraulic conductivity testing was conducted in natural conditions as well as under the simulated load of the waste (i.e., pressures of up to 800 kPa). This testing indicated a vertical hydraulic conductivity of  $2 \times 10^{-9}$  to  $3 \times 10^{-9}$  m/s under natural conditions, while under load, a decrease of approximately three orders of magnitude was observed ( $8 \times 10^{-12}$  m/s). The application of 800 kPa of pressure also resulted in a decrease in the void ratio of the material by two-thirds, suggesting that consolidation of the material will be relatively rapid in response to the increased loading pressure of the waste. This consolidation will result in a rapid increase in the tensile strength of the material; however it will also result in a substantial decrease in its permeability to the underlying sediments.

### 1.2.3 Silty Sand Till

A basal layer of silty sand till exists between the clayey silt layer and the bedrock. The till is locally gravelly, and possesses several discontinuous lenses and layers of sand throughout its profile. The material also appears more lacustrine in nature at sporadic



locales throughout the site. The thickness of this material underlying the bog at the landfill site, as determined by Fenco, ranges from approximately 2 to 30 m.

The hydraulic conductivity of the till was determined through field and laboratory testing to range from  $2 \times 10^{-5}$  to  $4 \times 10^{-8}$  m/s, with a geometric mean of  $8 \times 10^{-7}$  m/s. Average linear ground water velocity in this unit ranges between 1 and 50 m/year based on the results provided.

#### 1.2.4 Bedrock

The bedrock is granodioritic in composition (Barnett, *et al.*, 1991), and occurs at depths ranging from surficial exposure to greater than 20 m (based upon drilling and seismic monitoring) (Fenco, 1996). The rock quality designation (RQD) of the bedrock was noted by Fenco during coring of the bedrock at three locations throughout the area. RQD ranged from 63% to 100% within the three locations, suggesting that the bedrock is not highly fractured. Hydraulic conductivity testing was conducted by Fenco upon two wells drilled into the bedrock, indicating a range of  $4 \times 10^{-7}$  to  $9 \times 10^{-8}$  m/s, with a geometric mean of  $2 \times 10^{-7}$  m/s.

#### 1.2.5 Hydrogeology

Overburden ground water flow in the vicinity of the property is controlled by the bedrock topography. Active natural ground water flow is restricted primarily to within the till, with the bedrock forming a lower boundary to the overburden aquifer. As discussed above, the mean hydraulic conductivity of the till and bedrock (i.e. bulk hydraulic conductivity, K) units are  $8 \times 10^{-7}$  m/s and  $2 \times 10^{-7}$  m/s, respectively. The landfill is located near the top of a small, unnamed watershed. Ground water flow within this watershed is directed toward the northeast, parallel to the apex of a small bedrock valley (Figure 2). Flow rates range within the till range from 1 to 50 m/year. Leachate flow will be northeasterly with the migration of ground water toward an area of surface water existing immediately to the west of a culvert passing beneath Jones Road, at a distance of about 750 m northeast of the waste cells. Migration time of ground water from the landfill to the surface water within the peat is estimated at approximately 5 to 10 years. During the migration period, leachate contaminants will be attenuated and biodegraded.

#### 1.2.6 Hydrology

As discussed in Section 1.2.5, the site is near the top of a small, unnamed watershed that is situated between Crystal Bay (Silver Lake) to the north and Morgan Lake to the south. The watershed encompasses a total area of approximately  $4 \text{ km}^2$ , and ultimately discharges into Morgan Lake at a lateral distance of approximately 1.9 km southeast of the waste. Runoff from the area of the waste is directed primarily to the northeast through a bog/wetland complex toward a small surface water pond situated immediately





upgradient of the Jones Road culvert. Surface water flow then continues northeasterly through a series of channels, bogs, wetlands, and beaver ponds a further 700 m before finally shifting southward toward the eastern end of Morgan Lake (1,600 m). Morgan Lake lies within the larger Nelson River drainage system that encompasses the Lake Winnipeg River system and its tributaries, including the Little Black Sturgeon River, which flows both into and out of Morgan Lake.

To the northeast of the waste cells, a depression is noted in the topography (known as “the saddle”) that may allow for a portion of the surface drainage from the site to migrate into Crystal Bay during spring freshet or in response to large precipitation events. Crystal Bay is located approximately 350 m north of the waste cells.

Along the southern extent of the site parallel to Jones Road, a portion of the landfill property is situated within a second watershed to the south. Flow within this watershed drains southwesterly and ultimately into the western end of Morgan Lake (~450 m). To note, the boundary of this watershed lies greater than 50 m south of the waste cells.

## **2.0 SITE DESIGN & OPERATIONS**

The Jones Road landfill began accepting construction and demolition wastes on November 27, 2000 to provide a stable footprint for the waste cells. The site continued to collect these same waste materials through 2001 to 2009. As of November 18, 2009, the haul destination for all solid waste was revised from the Brady Road Landfill Site in Winnipeg, to the Jones Road Landfill. A summary of total annual quantities of waste received on a monthly basis at the Site over the period of 2012 - 2014 can be found in Appendix C.

The landfill has already filled sequence A-D, and is currently filling sequence E as per the SNC-Lavalin monitoring plan (September, 1999) (Figure 3), while the final contours are provided in Appendix L, which is from the Design and Operations Plan (Fenco McLaren, 1997). A 2013 Ministry inspection indicated there was deviation from the design plan, however it is noted that the City has since endeavoured to bring this back into compliance.

Based on scale readings from the incoming waste at the Site, the current waste mass as of December 2014 is 138,617 tonnes, or 17% of the total waste capacity (810,600 tonnes). The resulting remaining capacity for the Site based on average waste density and the final contour dimensions of the Site is 671,983 tonnes. Given the average annual waste



acceptance rate from 2010 to 2014 of 21,900 tonnes, there is a remaining lifespan for the Site of approximately 31 years.

The site possesses an approved waste footprint of approximately 13.1 ha (Figure 4). As noted in Section 1.1, the site operates under Ministry Certificate of Approval #A612018 (Appendix B). No complaints concerning the operation of the Jones Road Landfill Site were received by the City of Kenora over the period of 2012 – 2014.

### **2.1 Operation Changes**

The Jones Road Landfill is currently being operated and filled in accordance to the requirements established in the CoA as well as the Design and Operations Plan (Fenco McLaren. 1997). There have been no significant changes in operations, sequencing, equipment, or procedures made or produced at the Jones Road Landfill. No operating difficulties have been encountered over the course of 2012 – 2014. It is noted that the City is currently looking into establishing a composting operation at the Site to reduce the input of organic wastes (i.e. yard wastes, leaf, wood chips, etc). in 2014, the City submitted an application to include these operations at the Site, however, at the time of report issuance, no approval has been granted.

### **2.2 Cover Requirements, Erosion Protection**

Over the period of 2012 – 2014, cover material was added as necessary to aid in grading and site access. Dewatered sewage sludge was also added to provide a relatively level working surface. No erosion was noted in over the period of 2012 - 2014, which is expected given that the landfill is only in the infancy of its lifespan (i.e., the volume of material held within the waste cells is small and of little vertical height). Cover material and erosion protection material is inspected regularly by landfill staff.

## **3.0 SUMMARY OF 2012-2014 MONITORING PROGRAMS**

The 2012-2014 monitoring of ground water and surface water was facilitated through the collection of field measurements and water samples for laboratory analysis by City of Kenora staff. Ground water samples were collected in May and August, while surface water samples were collected in May, August and October. Sediment samples were collected in August of 2012.

The locations of the sampling stations monitored between 2012 and 2014 are depicted upon Figure 4. In May of 2013, the C of A was amended to reduce the frequency of sediment sampling from 5 to 10 years, and removed thirteen monitoring wells (10/7, 12/4, 6/14, 7/4, 8/22, 9/6, 2/17, 13/6, 13/14, 14/6, 14/21, 15/5, and 15/17) from the



sampling program. In addition, volatile organics were removed from the parameter list for all monitoring wells except for well 2/9. The above changes to the sampling program took effect at the start of the 2013 field season. The scope of the 2012 monitoring program was based on the requirements outlined in the previous MINISTRY Certificate of Approval. The details of the 2012-2014 monitoring program are summarized in Table 2 below.

**Table 1: Summary of the Required 2012-2014 Monitoring Programs**

Monitor Location	Annual Frequency	Parameters
<u>Ground Water (2012)</u>		
2/9, 2/13, 3/8, 4/6, 4/18, 5/17, 6/5, 6/14, 7/4, 16/15, 20/4, and 21/7	August	comprehensive list
2/9, 2/13, 3/8, 4/6, 4/18, 5/17, 6/5, 6/14, 7/4, 8/22, 9/6, 10/17, 11/4, 12/4, 13/6, 13/14, 14/6, 14/21, 15/5, 15/17, 16/15, 17/15, 19/16, 20/4, 21/7, 22/5, 23/3, KGS-2 and 24/5	May & August	indicator list & water levels
2/9, 2/13, 3/8, 4/6, 4/18, 5/17, 6/5, 6/14, 7/4, 8/22, 9/6, 10/17, 11/4, 12/4, 13/6, 13/14, 14/6, 14/21, 15/5, 15/17, 16/15, 17/15, 19/16, 20/4, 21/7, 22/5, 23/3, and 24/5	August	VOC's
<u>Ground Water (2013 - 2014)</u>		
1/17, 2/9, 2/13, 3/8, 4/6, 5/17, 6/5, 11/4, 16/15, 17/15, 19/16, 23/3, KGS-2 and 24/5	August	comprehensive list
1/17, 2/9, 2/13, 3/8, 4/6, 5/17, 6/5, 11/4, 16/15, 17/15, 19/16, 23/3, KGS-2 and 24/5	May & August	indicator list* & water levels
2/9	August	VOC's
<u>Surface Water (2012 – 2014)</u>		
SW-1, SW-2, SW-3, SW-4 and SW-5 (August only)	August & October	comprehensive list
SW-1, SW-2, SW-3 and SW-4	May	indicator list
SW-2 and SW-3	May, August & October	flow measurement
SW-2	August	VOC's
<u>Sediment (2012) **</u>		
SB-1, SB-2, and SB-R	August	major and minor inorganics & grain size

\* - Only locations where comprehensive samples are not required for the August event

\*\* - Sediment monitoring requirements were revised to every 10 years (next samples to be collected in 2021)



### **3.1 Monitoring Program Omissions**

#### **3.1.1 Ground Water**

Over the 2012 – 2014 monitoring period a number of monitoring well samples were not available for analysis. According to the accompanying field notes, it was indicated that over the period of monitoring, some wells were either out of service (i.e. damaged or not working) or did not contain an adequate supply of water for sample collection on one or more occasions.

In 2012, samples were not collected from MW4/6, MW4/18, MW20/4, and MW22/5 as the field notes indicated that the monitoring wells were out of service due to becoming damaged. It is noted that these wells have since been removed from the monitoring program and decommissioned, with the exception of MW4/6, which was re-constructed in the summer of 2013. Samples were also missing from MW5/17, MW11/4, MW10/17, MW13/6, MW14/21, and MW18/6 due to limited water or dry conditions. It is noted that these locations were subsequently removed from the monitoring program due to redundancy in data and / or the monitors continually being dry.

The following omissions were noted in 2013 and 2014 from monitors required as part of the revised monitoring program.

In 2013, dry conditions were also observed at MW5/17 & 11/4 for both sampling events, while no sample was collected at MW24/5 in August due to damaged sample tubing.

In 2014, samples were not collected at MW5/17 during the spring event due to a dry conditions and MW24/5 was not sampled during the spring event due to damaged tubing, which was subsequently repaired prior to the August monitoring event.

In all instances where dry monitors were encountered, similar conditions were observed historically.

#### **3.1.2 Surface Water**

Over the 2012 – 2014 monitoring period, some surface water samples were not able to be collected due to dry conditions during the sampling event. These include SW-3 in August of 2012/2013/2014, SW-5 in May 2012/2013, and SW-5 in October 2013.



### 3.2 Quality Assurance and Quality Control Results

As part of any routine sampling program, duplicate samples should be collected and analyzed for quality assurance purposes. Over the period of monitoring, six quality assurance/quality control (QA/QC) samples were collected. The results were within acceptable limits with only some minor exceptions which are summarized in Table 3.

**Table 2: Summary of Notable QA/QC Variations**

Location	Occasion	NH <sub>3</sub> -N	COD	TDS
MW16/15	May 2012		8.6 & 13	64 & 84
SW-5	August 2012			38 & 56
MW4/6	May 2014		74 & 100	

\* - all concentrations in mg/L

It should be noted that the results for the remaining parameters for each duplicate sample collected indicated very good correlation. It should also be noted that the inconsistencies observed were relatively small, within historical ranges and are notable mainly due to the very minimal and un-impacted concentrations at all duplicate locations. As such, although some inconsistencies are present, the general indication is that the majority of the results are consistent and generally acceptable. The variance in COD could be attributable to the fact that the bottles associated with these parameters are not field filtered. As such sediment within the sample bottles could result in some variance between sample bottles.

In the future, it is suggested that field staff ensure that all duplicate samples are “split” to minimize any potential natural variation in water chemistry.

### 3.3 Ground Water & Leachate (Schedule “F”)

During sampling over the monitoring period, water level measurements were obtained by City staff prior to any disturbance of the piezometric surface within each monitor using an electronic water level meter (accuracy of +/- 0.2 cm) as per Condition 2.4, Schedule F, of the current C of A. Ground water samples were then collected following purging of at least two borehole volumes of water from each monitoring well using dedicated check valve pumps and tubing. All ground water samples for inorganic analysis were also lab filtered.

Maxxam Analytics Inc. in Mississauga completed the analytical work from 2012 – 2014. The laboratory provided all sample bottles, which were prepared with preservatives for consistency, as required. Samples were maintained in coolers with freezer packs and



were delivered to the required laboratory within 24 to 36 hours of collection. A summary of the 2012 - 2014 analytical data is included in Appendix D.

### 3.3.1 Ground Water & Leachate Flow

Ground water measurements taken over the current monitoring period were compared to background data observed by Fenco during installation of several of the existing monitors in 1996. As detailed in Appendix E, in many locations the maximum ground water elevation value recorded over the 2012 -2014 monitoring period is below the value recorded in 1996. However in general the most recent water level data corresponds well with the available background data, indicating that ground water elevations in both the overburden as well as the shallow bedrock have remained relatively stable. Observed fluctuation in water table elevations over the course of 2012 - 2014 generally fall within 1 m, with a few exceptions.

Active ground water flow occurs within the till and is constrained by the bedrock surface, which forms a physical barrier to further vertical migration of ground water (i.e., like a bathtub). Within the bog, low topographic relief exists, as indicated by a surface elevation change of less than 2 m between the waste and the small pond at SW-1 (a total distance of ~750-800 m). For descriptive purposes, the water table elevations measured at all ground water monitors in May of 2014 have been included upon Figure 4. As can be observed, the slope of the water table corresponds well with the orientation of the local topography, with large lateral hydraulic gradients occurring at watershed boundaries (i.e., up to 0.24) and lower gradients occurring within the bog (i.e., as low as 0.001).

A watershed divide exists to the northeast of the waste footprint, between monitors 2 and 13 (see Figure 4). This divide is due to a rise in the elevation of the bedrock topography related to the surrounding topographic highs to the east, west, and north that exist beneath the bog. Within this watershed divide, there is an area which is referred to as the “saddle” area. The saddle represents a depression in a bedrock ridge that extends to the northeast. Monitoring wells are located in and around this feature (Figure 4 & 5) to determine ground water flow directions and gradients. With the exception of May 2007, ground water elevations within the saddle have been elevated above those to the south by at least 0.15 m since 2002, and was at least 0.45 m over 2012 - 2014 (Figure 5) indicating there is a limited potential for leachate migration past this feature. It is unclear as to why the gradient reversed during May 2007. It is also noted that as a result of Ministry concerns, more frequent measurements (at least once per month) were completed throughout 2010 and 2011 with results appearing to correlate well to the historical database (Figure 6).



### 3.3.2 Background Ground Water Quality

The background ground water geochemistry at the site is characterized by relatively low concentrations of most parameters (Table 5). Natural waters dissolve low quantities of elements through reaction with the soil minerals. Iron is derived through chemical weathering of soil and rock minerals and naturally occurs at levels approaching or greater than the Ontario Drinking Water Quality Standards (ODWQS). It is noted that elevated TDS concentrations were noted during both sampling events in 2014 with results greater than the ODWQS limit of 500 mg/L. As there were not corresponding elevations of any other parameter, it is possible, these results may be anomalous. As TDS is a general indicator to the presence of elevated inorganic constituents in the water, the minimal concentrations for the major ion chemistry does not match the elevated TDS concentration.

**Table 3: Background Chemistry**

	Ca	Mg	Na	Cl	Alk.	SO4	NH3-N	TKN	Fe	Cond.	TDS
<b>OWDS</b>	---	---	20 or 200	250	30-500	500	org N=0.15	---	0.3	---	500
<b>Overburden</b>											
# of samples	44	44	44	42	44	42	44	N/A	44	44	44
Maximum	37	10	11	17	130	11	6		13	248	828
Average	6.7	2.3	5.1	2.6	29	4.7	0.20		0.47	71	109
Minimum	2.0	0.8	2.0	0.5	9.0	0.5	0.03		0.01	31	10
STD	5.8	2.1	2.4	3.6	21.8	1.8	0.9		2.0	39	148
<b>Bedrock</b>											
# of samples	18	18	18	17	18	17	18	10	18	18	18
Maximum	26	5.2	4.3	8	91	10	0.12	1	0.54	216	184
Average	22	4	4	2	80	3	0.06	0.46	0.08	161	120
Minimum	14	3	3	1	53	1	0.03	0.06	0.01	139	92
STD	3.1	0.6	0.3	1.9	10.3	2.0	0.02	0.3	0.1	16.7	26.5

All values are given in mg/L. Overburden data are from monitors 11/4 and 12/4 and bedrock data is from monitor 5/17.

(STD = standard deviation, nd = not detected, n/a = not applicable, nt = not tested)

### 3.3.3 Leachate Quality

Leachate quality is controlled by the availability of soluble contaminants in the waste pile, the residence time of infiltrating water in the waste, and the physical conditions, such as temperature, redox potential, and pH of the solution. Compared to background waters, leachate that is produced from landfill waste typically possesses elevated concentrations (x10 or more) of magnesium, sodium, potassium, iron, zinc, chloride, alkalinity, ammonia, total kjeldahl nitrogen, conductivity, total dissolved solids and phenols. Since municipal wastes have only been accepted for a short time, leachate has not been created that will be substantially different than background waters. The acceptance of de-watered sewage sludge and contaminated waste from the removal of fuel tanks as well as the construction, demolition and industrial waste the site received to date may affect the parameters in the leachate and downgradient water samples. High



concentrations of total suspended solids (TSS) are explained by the lack of filtering of the general chemical samples in the field.

The quality of ground water underlying the waste has remained consistent to very slightly elevated (for certain inorganic compounds such as calcium and alkalinity) compared with the background ground water geochemistry. ODWQS was noted to be exceeded for alkalinity, DOC, iron, manganese, TDS at a few locations in or immediately surrounding the active waste area (MW23/3, 24/5, KGS/2, 1/17). Although most likely associated with leachate generation, these concentrations are not viewed as a concern at this point given their relatively low and inconsistent concentrations, as well as the fact they are not seen to be elevated at any downgradient location owing to leachate presence.

#### 3.3.4 Downgradient Ground Water Quality

As in previous years, the 2012 - 2014 analytical data do not indicate that leachate generation from the waste has resulted in impact to ground water quality either immediately downgradient of the waste or at any of the other monitors installed within the Contaminant Attenuation Zone (CAZ). Since the landfill commenced operation in the spring of 2001, the concentrations of parameters at all downgradient monitoring locations have remained at or very slightly above background levels, as can be observed in the time series graphs provided in Appendix F.

A suite of volatile organic compounds (VOC's) was analyzed at selected locations during sampling in August 2012 as per Condition 2.3(b) of Schedule F, while samples in 2013 and 2014 were limited to MW2/9 and SW-2. No detections for any parameters were noted during this monitoring period. A summary of detected VOCs between 2001 and 2014 can be found in Appendix G.

### 3.4 Surface Water (Schedule "C")

City staff collected surface water samples in May, August and October of 2012, 2013, and 2014, with Maxxam Analytics Inc. in Mississauga completing the analytical work. The laboratory provided all sample bottles, which were prepared with preservatives for consistency, as required. The samples were kept in coolers with freezer packs and were delivered to the laboratory within 24-36 hours of collection. The 2012-2014 analytical data have been summarized and are included in Appendix H.





### 3.4.1 Surface Water Flow

Surface water from the area of the waste flows in a northeasterly direction through a large bog/wetland complex before eventually discharging into a pond at the west side of the Jones Road Culvert (SW-1) that serves as the headwaters of an unnamed creek. Discharge into the pond from the bog includes both surface flow and interflow within the peat.

From the pond, the creek flows beneath the roadway and in a northeasterly direction for approximately 700 m, before turning southward and eventually discharging into a further bog/wetland complex south of a logging road about 800 m upgradient of Morgan Lake (SW-2). Along its route, the creek is discontinuous as it passes through various bogs, wetlands, and beaver ponds. Periodic discharge from the Lunch Lake sub-watershed commingles with flow in the creek, providing additional flow at SW-2.

Approximately 150 m northeast of the waste footprint, a depression is noted in the landscape, known locally as the “saddle”. This area allows a portion of surface drainage to migrate into Crystal Bay during spring freshet or in response to large precipitation events. Crystal Bay is located approximately 350 m north of the waste cells. Figure 2 shows the site’s hydrologic features.

Estimates of stream flow were made by City Staff at SW-1, SW-2, SW-3, and SW-4 during the May and October sampling periods and at SW-1, SW-2, SW-3, SW-4 and SW-5 during August, as per Condition 2.4, Schedule C of the current C of A. Flow velocity measurements are summarized in the following table. As in previous years, flows are shown to be low and intermittent.

**Table 4: Surface Water Flow Data**

Location	Velocity (m/sec)								
	2012			2013			2014		
	May	Aug	Oct	May	Aug	Oct	May	Aug	Oct
SW-1	0.18	0	0.24	0.18	0	0	0	0	0
SW-2	0.06	0.06	0.24	0.30	0.03	0.06	0.11	0.03	0.3
SW-3	0	0	0	-	0	0	0	0	0
SW-4	0	0	0	-	0	0	0	0	0
SW-5	-	0.02	-	-	0	-	-	0	-



Although the flow data collected is limited, it is apparent that there is a consistent increase in flow between SW-1 and SW-2. This increase in flow would indicate that surface water contributions outside of the landfill property are providing attenuation through an increased dilution potential in the downstream feature.

#### 3.4.2 Surface Water Quality

Surface water quality data obtained over 2012 -2014 were compared to the background quality data obtained at SW-1 and SW-2 prior to the construction of the site in 2000, as well as to the Provincial Water Quality Objectives (PWQO). In general, the surface water quality is good and falls within the limits of the PWQO, with the exception of iron, total phosphorus, and phenols. However, it was noted that concentrations are consistent with background concentrations and likely indicative of the shallow and stagnant nature of these locations where sediment entrainment may cause elevations of these parameters, especially iron, while natural decay of organic materials is attributed to the elevated total phosphorus concentrations.

As no impacts were observed at any ground water monitoring locations downgradient of the waste, and the composition of waste currently deposited at the site would not have generated a plume of sufficient size and strength to affect these locations, given the short residence time at the site, natural sources are believed to be the most likely source.

Boron values have been steadily increasing at SW-1 over the past ten years. Elevated levels of boron above the PWQO began as an isolated value in 2008, however it has been consistently measured above the PWQO at SW-1 over the period of 2012 – 2014. Although these values are increasing, they are not yet above the action level of 1.0 mg/L according to the trigger mechanism in place. Boron values at SW-1 should be carefully watched going forward and any potential trigger exceedances need to be reviewed relative to other leachate indicator parameters as well as the downstream concentrations (SW-2).

A time-trend analysis of selected leachate parameters at SW-1 since 1996 is included in Appendix I. The developing geochemical database is illustrating a slight increasing trend for select typical leachate related parameters including alkalinity, conductivity, boron and sulphate. The elevations of these parameters specifically are not surprising as they are reflective of construction and demolition wastes (i.e. gypsum [calcium sulphate] wallboard). The boron is likely derived from boric acid, which is added to gypsum board to increase the strength of the material, reduce weight and provide better adhesion of the paper to the board. With the addition of municipal wastes over the last few years, a close watch will be kept on other parameters such as chloride and ammonia, which are good



indicators or municipal wastes. As most leachate indicator parameters have remained at relatively low concentrations, these values are not seen as an issue at this time. As well, these parameters have limited potential for environmental impacts unless they reach very high levels. A similar time-trend analysis for SW-2 indicates the same general trend observed at SW-1 has begun in 2008, however to a much smaller degree. These increasing trends indicate that possible impacts from construction, demolition and industrial waste fill are most apparent in the area closest to the waste area (SW-1), while impacts are reduced further away from the footprint, indicating attenuation. This is most evident in the boron concentrations which exceed PWQO at SW-1, but are an order of magnitude lower at SW-2. SW-3 is a background location that receives water from the Lunch Lake watershed and it shows increased ranges as well, although not to the same degree.

SW-4 & SW-5 are further downstream locations which have not surprisingly shown no measurable leachate impacts indicating that attenuative processes downstream of the landfill continue to be active and sufficient to address surface impacts observed a short distance downstream of the landfill SW-1.

### 3.5 Trigger Mechanism Assessment

The Trigger Mechanism and Contingency Plan for the Site was revised and submitted to the Ministry in 2014 for their review and comment. The purpose of this update is to provide a more appropriate trigger mechanism approach based on the current data set which now has 12 years of seasonal geochemical data. Leachate quality data is now better understood with the landfill recently beginning to accept municipal wastes in 2009.

Similar to the original plan, both surface and ground water monitoring locations were targeted. The targeted parameters and associated trigger concentrations for ground water are summarized in the following table.

**Table 5: Proposed Trigger Limits – MW3/8**

Parameter	ODWQS	Background *	Trigger Limit at MW3/8 **
Chloride	250	2.2	126
TDS	500	147	324
Alkalinity	500	68	284

It is noted that all concentrations at MW3/8 for these parameters have been less than 75% of the associated criteria for the period of record, with all concentrations for the reporting period (2012-2014) noted to be less than 50%.



Similar trigger limits were also created for surface water at SW-2, as well as an action criteria at SW-1, which is in place to provide additional review of the geochemical data to assess any potential trends prior to trigger limits being exceeded at SW-2. The criteria established is summarized in the following table.

**Table 6: Proposed Surface Water Trigger Limits**

Parameter	Typical Landfill Leachate	SW Background (SW-3)	Action Level at SW-1	Trigger Limit at SW-2*
Chloride	1000	1.9	158	187
Boron	5	0.02	0.8	0.2
Ammonia (unionized)	5	0.0005	0.8	0.02

It is noted that both chloride and un-ionized ammonia have both show to be well below their associated criteria at both locations (<45%), while boron has shown to be much more elevated (as discussed in Section 3.4), with concentrations (0.81 mg/L – August 2014) near the action level (1.0 mg/L) at SW-1. Although slightly elevated at SW-2 (0.1 mg/L – August 2014), the boron concentrations are shown to be greatly reduced from SW-1, indicating attenuative processes are active. As well, given the other leachate indicator parameters are not showing similar elevated levels in the surface water at SW-1 or SW-2, the elevated boron is not reflective of a large scale leachate trend. However, these trends will be monitored closely over the next three years as the leachate signature at the site evolves with an increased waste volume.

### 3.6 Sediment (Schedule “D”)

In 2012, City staff collected sediment quality samples on August 31<sup>st</sup> at three locations as specified in the current C of A (i.e., SB-1, SB-2 and SB-R). As noted previously, in 2013, the frequency of sediment sampling was adjusted to every ten years. After 2012, the next sediment sampling event is not required until 2021. The 2012 analytical data have been summarized in Appendix J.

The analytical data obtained from each of the samples were compared to the criteria outlined in, *Guidelines for the Protection and Management of Aquatic Sediment Quality in Ontario* (MOE, 1993). Of the seventeen required parameters, not a single parameter exceeded in 2012 for either the lowest effect level (L.E.L.) or severe effect level (S.E.L.).

Grain size analysis was performed on the sediment samples in 2012. The results are located in Appendix J. Samples indicate that the SB1 and SBR are sands, while SB2 is noted to be silt. The results are somewhat similar to those found in previous years.



### **3.7 Biological Effects (Schedule “E”)**

Benthic invertebrate monitoring was conducted from 2000 to 2003. The need for continued or additional benthic monitoring is not necessary, since the minimum of three (3) years of sampling, as stated in the C of A, has been fulfilled. As requested during the 2013 Ministry inspection, the data from these previous studies has been included as an appendix for completeness (Appendix .

### **3.8 Landfill Gas (Schedule “G”)**

Landfill gas was previously required according to the C of A Schedule “G” on a bi-monthly basis during frozen ground conditions and quarterly during all other periods at the two proposed landfill gas monitors outlined in the SNC-Lavalin monitoring plan (September, 1999). Landfill gas was removed from the monitoring program as documented in the Ministry document dated April 23, 2013, however, as required in the letter, the monitoring locations were maintained for future use.

Although the potential for methane generation has increased with the commencement of domestic waste deposition at the Site in 2009, the limited size of the waste mound and relative small waste acceptance rate would still limit the methane generation at the Site. As well, the geochemical signature of the landfill leachate, which is most notable at MW1/17 (downgradient edge of waste) and MW23/3 (middle of waste) still has a relatively weak and variable leachate signature with chloride concentrations less than 150 mg/L. Elevated ammonia and DOC concentrations are also developing, which indicate anaerobic conditions are beginning to develop, likely the result of the acceptance of domestic wastes at the Site. Given this newly developing trend, significant methane production has not likely yet developed at this point, however, will be assessed as part of the next annual report, which is to be submitted in 2018. This time frame is deemed to be appropriate given the remoteness of the Site limits the potential for risk to surrounding residents. However, if a significant increasing trend in leachate concentrations develops in the interim, the City will conduct a round of methane measurements to document landfill gas concentrations such that an assessment regarding re-instatement of measurements as part of the monitoring program is necessary.

### **3.9 Geotechnical Monitoring (Schedule “H”)**

Shear testing was completed by Eng-Tech Inc. in 2014 within the east side of Cell A. The purpose of the testing was to determine the in-situ shear strengths and pore water pressures along the east side of the landfill in order to confirm previous geotechnical information obtained in 2009. Eng-Tech drilled four (4) boreholes along/ Eng-Tech also



installed two (2) vibrating wire piezometers within each borehole as per Schedule H of the C of A.

According to the Eng-Tech report, the in-situ shear strength values obtained in 2014 ranged from 20 to 31 kPa and are above the minimum recommended 5.5 kPa value by R. Kerry Rowe Inc. in their report *Kenora Landfill* dated February 21, 1997, Eng-Tech concluded that based on the soil stratigraphy and the shear strength values obtained on site, the 2014 results are in compliance with the values as stated in the previous geotechnical evaluation in 2009. Eng-Tech also supports the loading as recommended in the R. Kerry Rowe Inc (1997).

The complete Eng-Tech report can be found in Appendix K.

#### 4.0 PROPOSED 2015 -2017 MONITORING PROGRAM

It is recommended that the 2015 – 2017 monitoring program for the City of Kenora Jones Road Landfill site remains the same as the 2014 program, which should follow the current C of A No A612018. The 2015 – 2017 program includes the monitoring of ground water levels and collection of ground and surface water samples as outlined in the following table.

**Table 7: Summary of the Required 2015-2017 Monitoring Programs**

Monitor Location	Annual Frequency	Parameters
<i>Ground Water</i>		
1/17, 2/9, 2/13, 3/8, 4/6, 5/17, 6/5 and 16/15	August	comprehensive list
1/17, 2/9, 2/13, 3/8, 4/6, 5/17, 6/5, 11/4, 16/15, 17/15, 19/16, 23/3, KGS-2 and 24/5	May & August	indicator list* & water levels
2/9	August	VOC's
<i>Surface Water</i>		
SW-1, SW-2, SW-3, SW-4 and SW-5 (August only)	August & October	comprehensive list
SW-1, SW-2, SW-3, SW-4 and SW-5	May	indicator list
SW-2 and SW-3	May, August & October	flow measurement
SW-2	August	VOC's

\* - Only locations where comprehensive samples are not required for the August event



As per the Ministry letter dated April 24, 2013 (Appendix B), sediment sampling is to remain in the monitoring program, however the frequency of sample collection has been reduced to every 10 years with the next sample being required in 2021. However, it is also stated that if annual surface water samples in Morgan Lake or Crystal Bay show a significant increase in metal concentrations then the sediment sampling should be repeated in 2016 and thereafter every five years. Based on the information outlined in Section 3.4, there does not appear to be a significant increase in metal concentrations, so it is therefore recommended that sediment sampling continue with the current sampling frequency of once in every ten years. Based on this information, the next sediment sample collection should occur during the 2021 field season.

## **5.0 CONCLUSIONS AND RECOMMENDATIONS**

Operation of the City of Kenora Jones Road Landfill site is in compliance with the C of A and is performed as designed. The site is not creating any adverse impacts on the surrounding environment. Therefore there are no requirements for any mitigation / remedial measures based on the existing site performance data compilation.

Currently, limited impacts to the ground water regime are observed only within either the landfill footprint or at monitors situated immediately downgradient of the waste cells. In addition, only minor impacts were observed at SW-1 (closest to fill area), which are assumed to be a result of the construction/demolition and sewage sludge waste and the recent addition of municipal waste. Slight water quality impacts were also measured at SW-2 and SW-3 although to a much lesser degree than observed at SW-1.

## **6.0 REFERENCES**

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Azimuth Environmental Consulting, Inc. 2009. *2008 Annual Monitoring Report, Jones Road Landfill Site.*

Azimuth Environmental Consulting, Inc. 2008. *2007 Annual Monitoring Report, Jones Road Landfill Site.*



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Barnett, P.J. Cowan, W.R. and Henry, A.P. 1991. *Quaternary Geology of Ontario, northern sheet*; Ontario Geological Survey, Map 2556, Scale 1:1,000,000.

DST Consulting Engineers Inc. 2001. *City of Kenora – 2001 Landfill Monitoring Program.*

Fenco McLaren. 1997. *Kenora Area Waste Management Master Plan – EPA Studies Document.*

Ministry of the Environment. 1993. *Guidelines for the Protection and Management of Aquatic Sediment Quality in Ontario.*

Ministry of the Environment. 1994. *Provincial Water Quality Objectives.*

Ministry of the Environment. 2000. *Ontario Drinking Water Standards.*

Wardop Engineering, Ltd. 2001. *Kenora Area Solid Waste Landfill Site – Annual Report.*





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## APPENDICES

- Appendix A: Figures**
  - Appendix B: MOE Correspondence and Certificates of Approval**
  - Appendix C: Annual Waste Quantities**
  - Appendix D: Ground Water Quality Data**
  - Appendix E: Ground Water Levels**
  - Appendix F: Ground Water Chemistry Over Time Graphs**
  - Appendix G: Detected Volatile Organic Compounds**
  - Appendix H: Surface Water Quality Data**
  - Appendix I: Surface Water Quality Over Time Graphs**
  - Appendix J: Sediment Sampling Data**
  - Appendix K: Eng-Tech 2014 Report**
  - Appendix L: Final Contour Design Drawing**
  - Appendix M: Borehole Logs**
  - Appendix N: Benthic Invertebrate Monitoring Data**
  - Appendix O: MOE Landfill Reporting Submission Forms**
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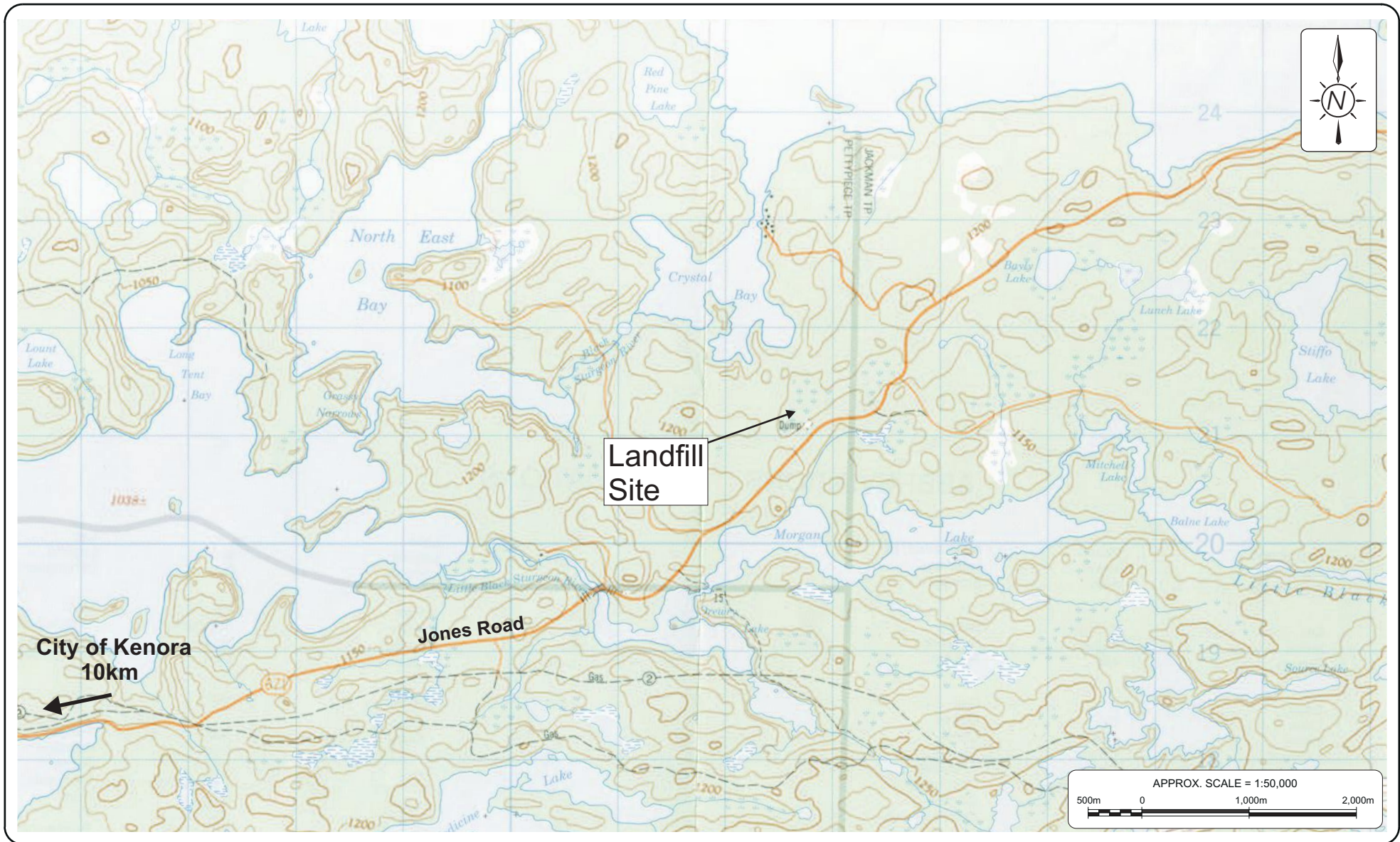
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## **APPENDIX A**

### **Figures**

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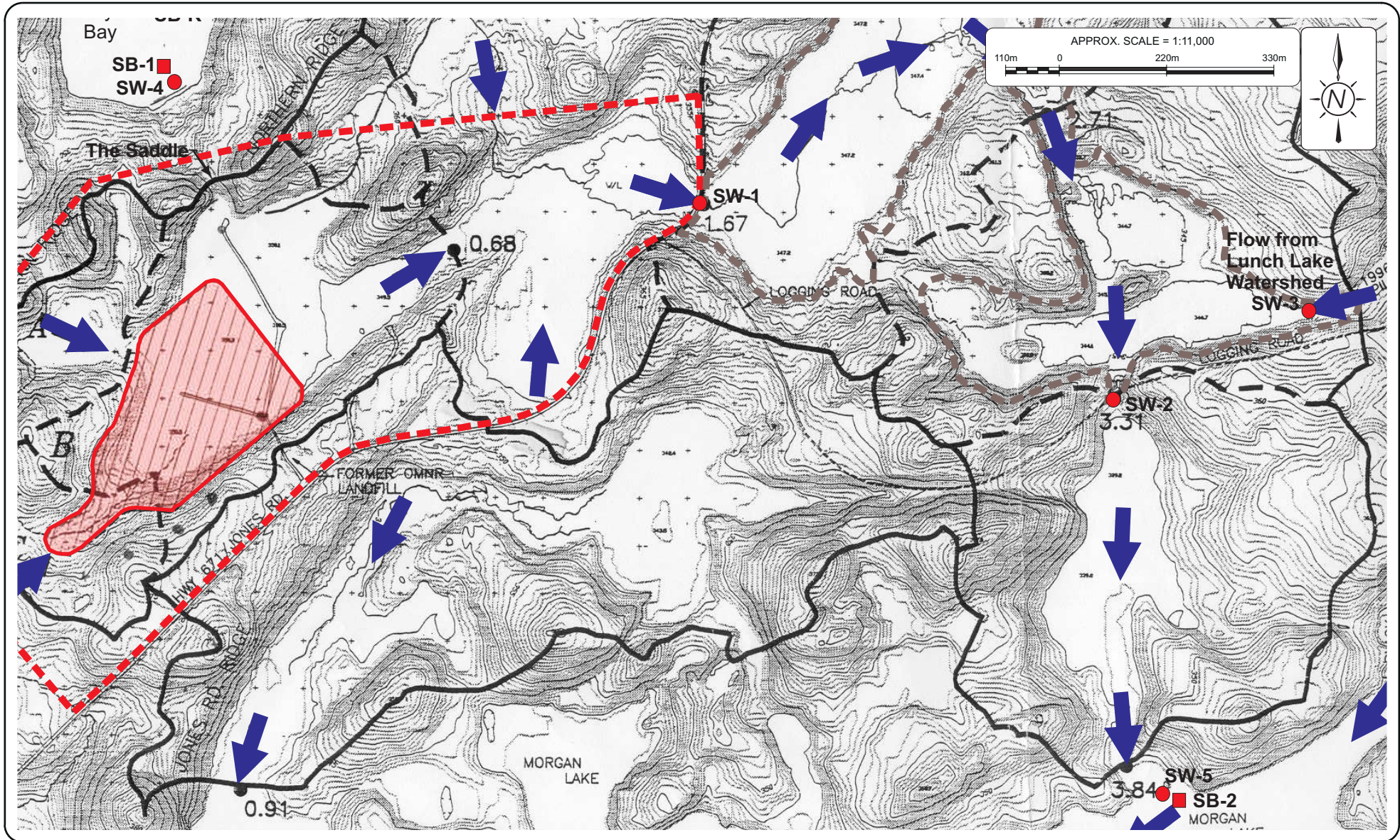
Legend:



### Site Location

Date Issued:	June 2015	2015 Annual Monitoring Report Jones Road Landfill Site City of Kenora	Figure No. <b>1</b>
Created By:	CMR		
Project No.:	15-020	MOE Certificate of Approval No. A612018	
File Name:	Figure 1 - Site Topo.CDR		

Source: Energy, Mines, and Resources Canada. 1996. 1:50,000



**Legend:**

- Approved Waste Footprint
- Approximate Property Boundary
- Contaminant Attenuation Zone
- Watershed Boundary
- Surface Water Sampling Location
- Sediment/Benthic Sampling Location
- Surface Water Flow

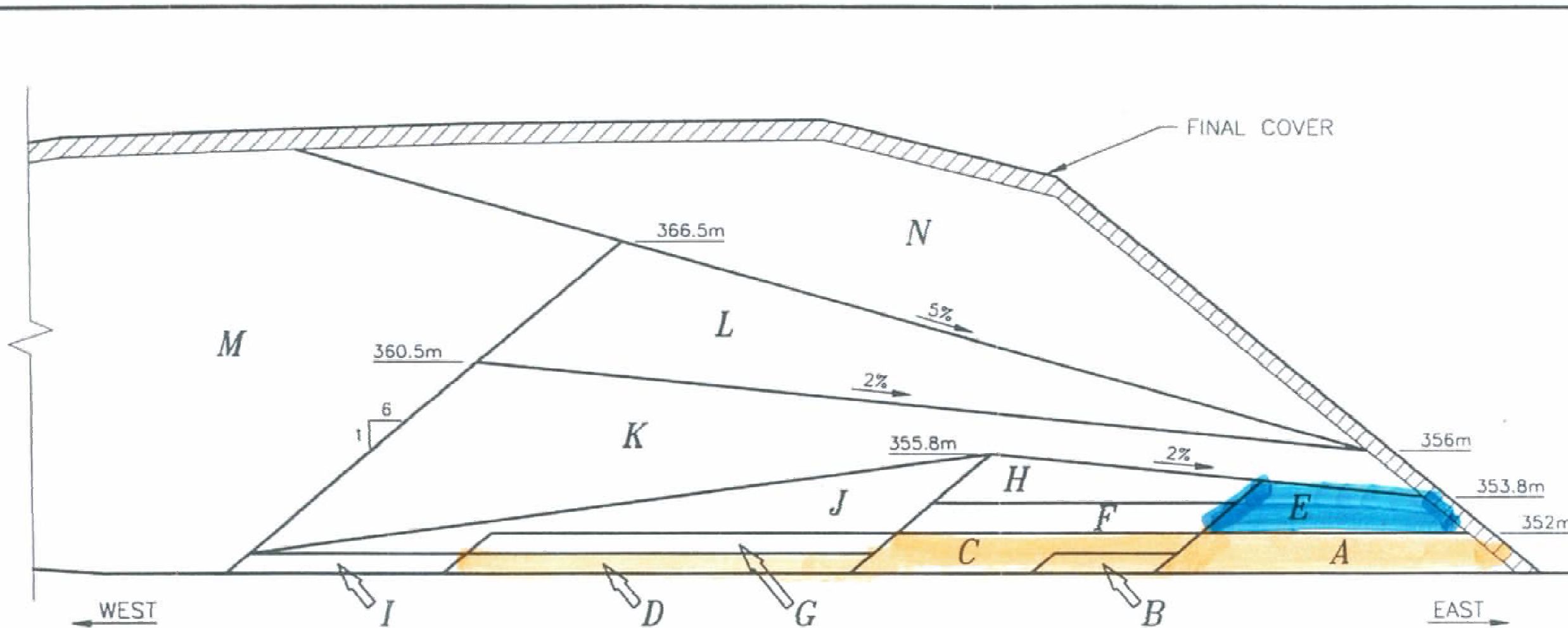


**Hydrologic Features**

<b>Date Issued:</b> June 2015	<b>2015 Annual Monitoring Report</b>	<b>Figure No.</b>
<b>Created By:</b> CMR	<b>Jones Road Landfill Site</b>	<b>2</b>
<b>Project No.:</b> 15-020	City of Kenora	
<b>File Name:</b> Figure 2 - Hydrologic Features.CDR		
		<b>MOE Certificate of Approval No. A612018</b>

Base Drawing: Fenco MacLaren (1997), Figure S3.1

LEGEND:



LANDFILL SEQUENCE	APPROX. YEAR OF DEVELOPMENT	LANDFILL SEQUENCE	APPROX. YEAR OF DEVELOPMENT
A	1 - 3	H	11
B	4	I	12
C	5	J	13
D	6	K	14 - 20
E	7 - 8	L	21 - 23
F	9	M	24 - 35
G	10	N	36 - 40

*Existing sequence of fill*

*already filled.*

**Fenco MacLaren**  
  
 KENORA AREA  
 WASTE MANAGEMENT MASTER PLAN  
 PROPOSED KENORA AREA LANDFILL

**LANDFILL DEVELOPMENT SEQUENCE**  
 SCALE HOR. 1 : 1500  
 VERT. : 300

Not to Scale

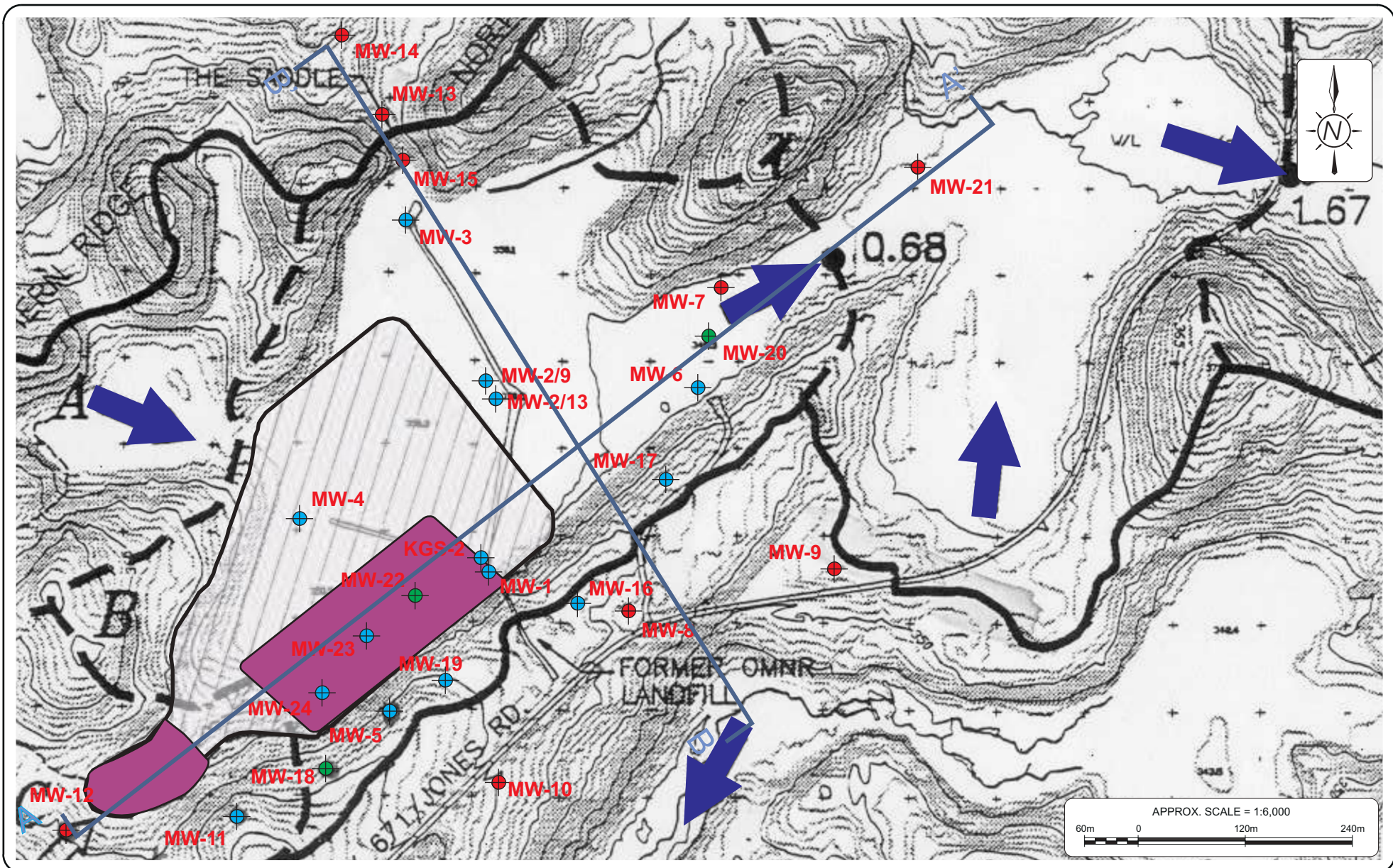


Landfill Development Sequence







Jones Road Landfill  
 City of Kenora

DATE ISSUED: June 2015	Figure No.
CREATED BY: JLM	3
PROJECT NO.: 15-020	
REFERENCE:	

Plotted by: MCCARTNEY on June 2, 2015 at 2:07pm  
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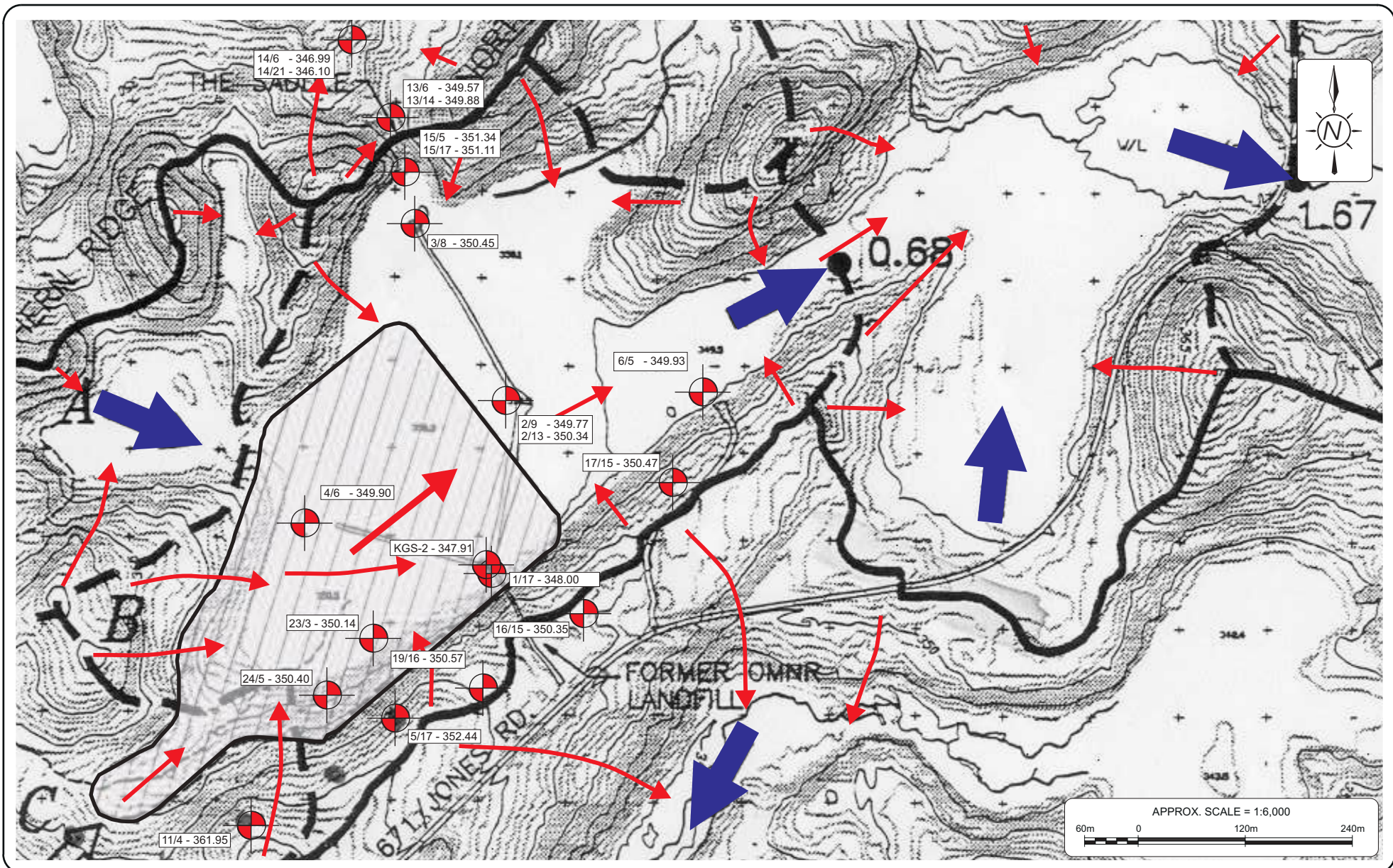
-  Approved Waste Footprint
-  Ground Water Monitor (maintained)
-  Ground Water Monitor (removed from program)
-  Ground Water Monitor (decommissioned)
-  Watershed Boundary
-  Active Footprint Area

Base Drawing: Fenco MacLaren (1997), Figure S3.1









**Ground Water Monitoring Network**

Date Issued:	June 2015	2015 Annual Monitoring Report Jones Road Landfill Site City of Kenora	Figure No.
Created By:	CMR		<b>4</b>
Project No.:	15-020		
File Name:	Figure 4 .CDR	MOE Certificate of Approval No. A612018	



**Legend:**

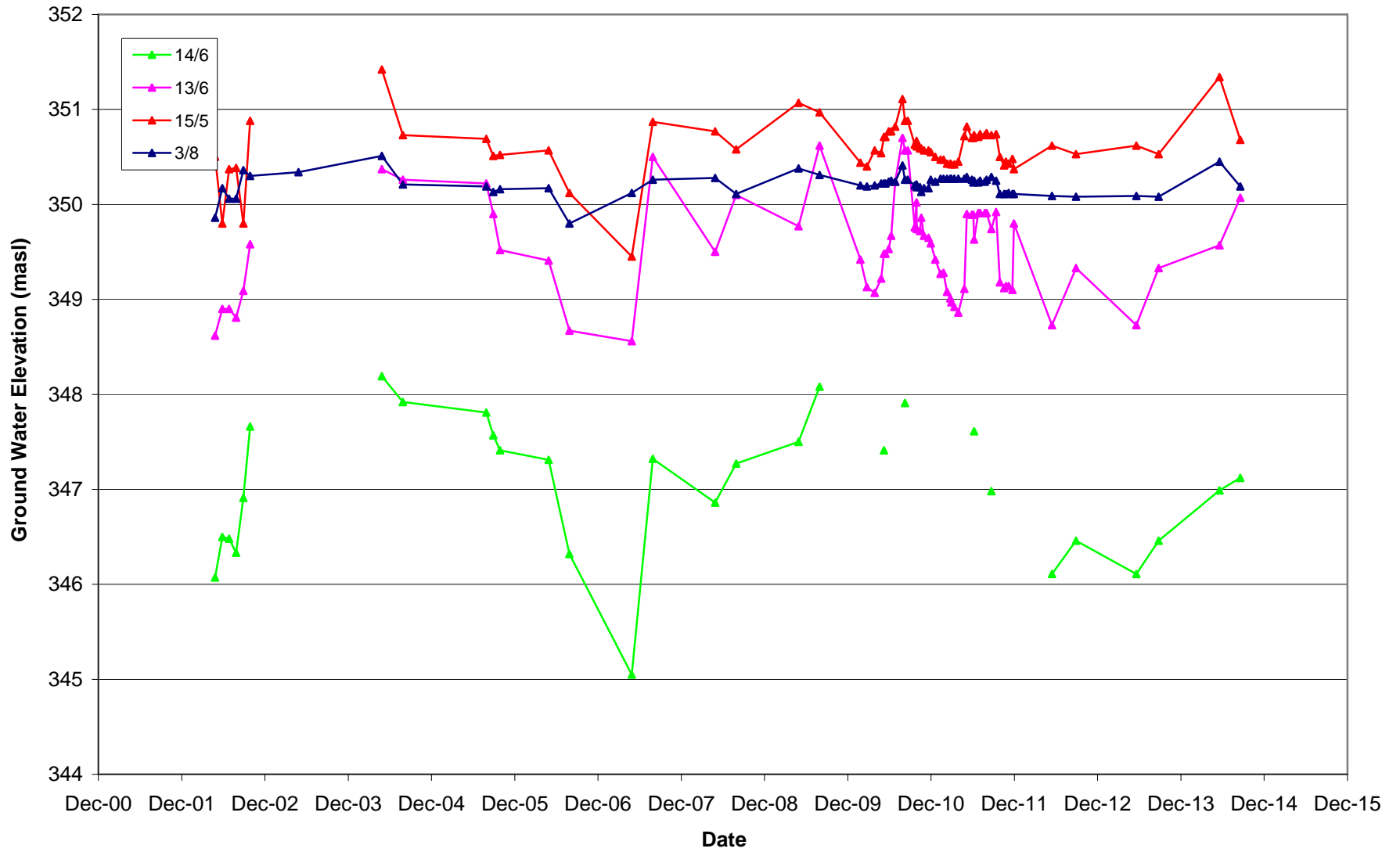
-  Approved Waste Footprint
-  Ground Water Monitor
-  Watershed Boundary
-  Ground Water Flow Direction
-  Surface Water Flow Direction
-  Sub-watershed Boundary



**Ground Water Monitoring (May 2014)**

Date Issued:	June 2015	2015 Annual Monitoring Report Jones Road Landfill Site City of Kenora	Figure No. <b>5</b>
Created By:	CMR		
Project No.:	15-020		
File Name:	Figure 5.CDR		
		MOE Certificate of Approval No. A612018	

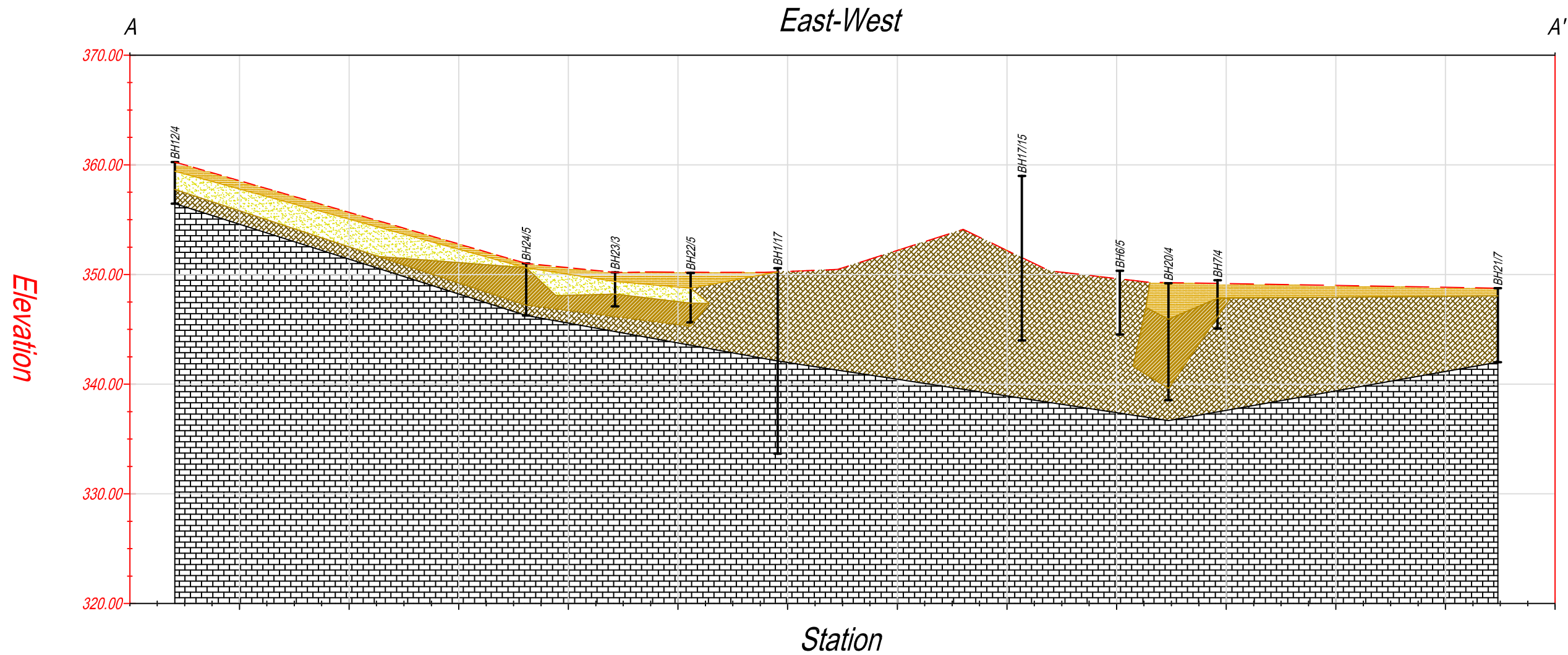
Figure 6 - Saddle Ground Water Elevations (overburden)





LEGEND:

-  Bedrock
-  Peat
-  Till
-  Sand
-  Clay






East-West Cross Section A-A'

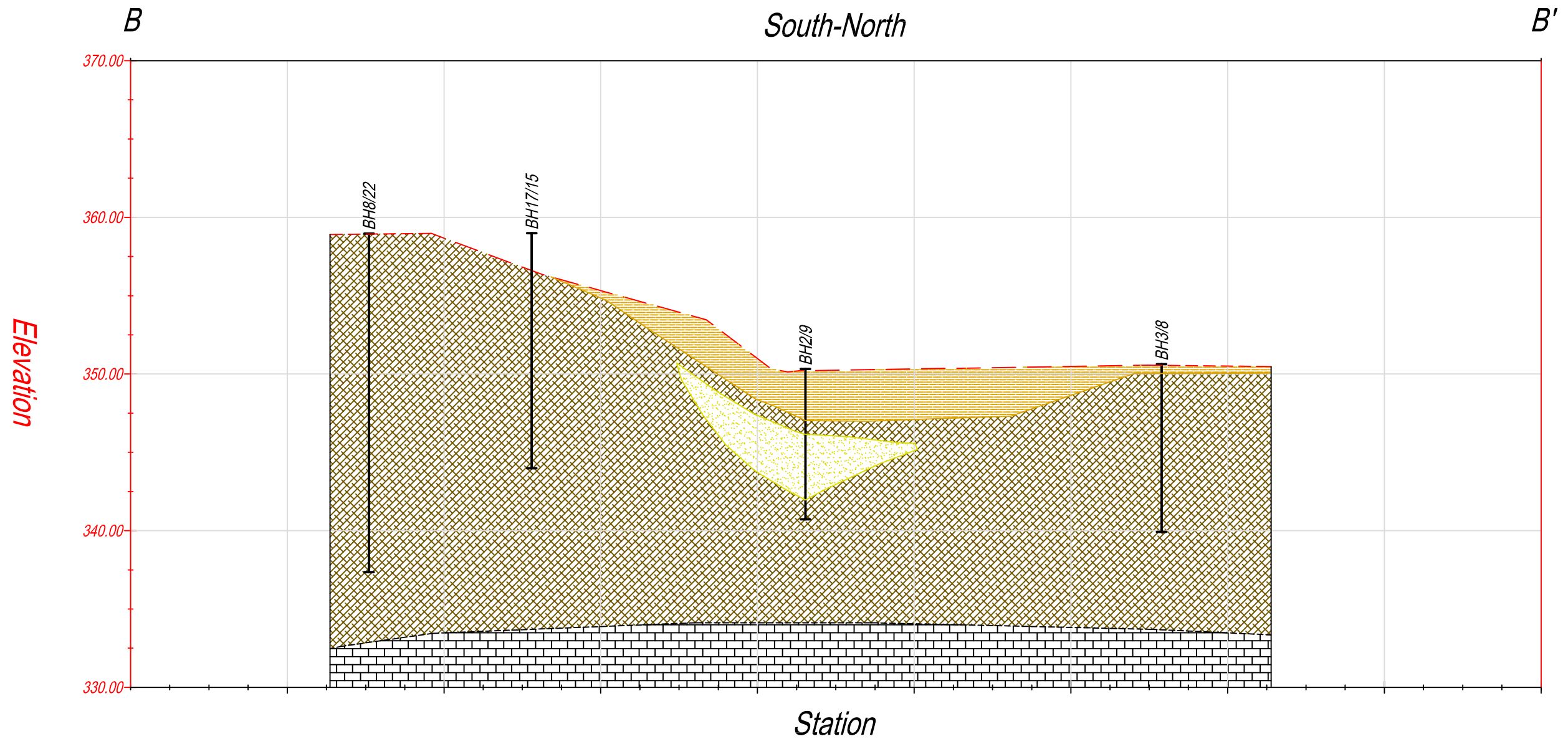
Jones Road Landfill  
City of Kenora

DATE ISSUED:	June 2015	Figure No.
CREATED BY:	JLM	7
PROJECT NO.:	15-020	
REFERENCE:		

Plotted by: MCCARTNEY on June 2, 2015 at 1:23pm  
File: M:\15 Projects\15-020 2015 Jones Road Landfill Reporting\04.0 - Drafting\15-020.dwg Layout: Figure 7 Plotscale: 0.1

LEGEND:

-  Bedrock
-  Peat
-  Till
-  Sand



South-North Cross Section B-B'

Jones Road Landfill  
City of Kenora

DATE ISSUED: <i>June 2015</i>	Figure No.
CREATED BY: <i>JLM</i>	8
PROJECT NO.: <i>15-020</i>	
REFERENCE:	

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**APPENDIX B**

**MOE Correspondence and Certificates of Approval**

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**APPENDIX C**

**Annual Waste Quantities**

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## 2014 Waste Values

	Waste Volume (tonnes)												
	Jan	Feb	Mar	Apr	May	Jun	Jul	Aug	Sept	Oct	Nov	Dec	TOTAL
<b>Collected Material</b>													
Garbage	616.5	442.7	551.5	638.5	764.4	644.5	678.5	666.8	601.7	647.8	503.0	660.6	7416.6
Construction and demolition waste	253.0	338.6	407.4	467.7	1122.7	1055.7	1022.4	972.9	1,223.8	866.6	570.5	1,257.6	9,558.9
Contaminated waste	0.0	643.3	2.5	0.0	15.6	2.3	23.9	0.0	562.4	0.3	0.0	0.0	1,250.4
Sewage sludge	196.3	223.0	293.6	301.7	298.0	195.1	207.6	133.8	208.2	198.0	242.2	179.8	2,677.1
Ash	264.6	223.7	268.2	314.4	279.5	248.2	236.1	251.5	262.0	190.2	260.4	286.4	3,085.1
Asbestos	0.9	0.6	0.0	1.0	0.4	7.8	0.0	0.0	0.9	0.4	4.3	1.2	17.4
Woodchips	0.9	0.0	0.5	13.1	46.0	37.2	61.3	45.7	33.3	44.7	8.8	1.1	292.5
												<b>Collected TOTAL</b>	<b>24,298.0</b>
<b>Diverted Material</b>													
Recycles	150.9	100.8	112.6	126.7	140.5	128.2	119.7	130.2	134.9	129.3	111.2	107.0	1,492.0
Scrap metal	5.9	9.2	4.6	12.3	25.1	17.9	16.9	20.3	14.0	13.6	7.2	5.8	152.9
Electronics	4.9	10.0	0.0	5.1	10.7	19.7	14.2	5.3	10.3	8.9	12.8	9.0	110.8
												<b>Diverted TOTAL</b>	<b>1,755.7</b>
												<b>GRAND TOTAL</b>	<b>26,053.7</b>

Municipal Solid Waste quantities collected in tonnes for 2013

	January	February	March	April	May	June	July	August	September	October	November	December	Total
<b>Garbage</b>	553	492	501	655	768	780	868	887	653	672	556	585	7,970
<b>C &amp; D</b>	333	295	339	446	644	691	714	709	832	935	850	248	7,035
<b>Recycles</b>	140	94	109	126	110	129	168	137	121	135	104	94	1,468
<b>Scrap metals</b>	3	3	4	9	21	24	18	19	18	12	9	4	144
<b>Contaminated Waste</b>	0	3	6	41	398	0	9	58	9	1,507	0	0	2,030
<b>Sewere sludge</b>	211	204	241	192	286	132	237	194	204	268	130	149	2,447
<b>Ash</b>	238	305	262	298	282	180	214	245	199	254	200	258	2,934
<b>Electronics</b>	5	5	6	0	5	14	15	5	11	0	21	5	93
<b>Asbestos</b>	0	0	1	0	0	0	3	1	0	1	2	4	13
<b>Woodchips</b>	1	1	1	7	50	44	76	47	33	45	9	1	313
<b>Total</b>	<b>1,484</b>	<b>1,403</b>	<b>1,470</b>	<b>1,773</b>	<b>2,565</b>	<b>1,994</b>	<b>2,321</b>	<b>2,300</b>	<b>2,080</b>	<b>3,829</b>	<b>1,880</b>	<b>1,347</b>	<b>24,445</b>

Solid Waste Quantities 2012 in Tonnes

Month	Garbage	C& D	Recycles	Scrap Metals	Contaminated Waste	Sewarage sludge	Ash	Electronics	Asbestos	Wood chips	Total
January	508	277	110	8	149	173	157	7	0	0	1,390
February	436	323	122	7	136	221	256	0	0	1	1,501
March	570	692	134	10	30	188	245	5	0	11	1,885
April	554	737	88	17	103	161	250	0	0	32	1,943
May	669	618	135	15	15	238	239	6	0	50	1,985
June	682	725	130	13	9	228	184	6	0	66	2,043
July	793	656	149	18	0	143	169	0	0	41	1,969
August	841	626	162	17	19	183	173	18	1	43	2,084
September	561	791	127	12	93	138	243	6	0	39	2,009
October	644	891	137	15	0	208	264	0	0	30	2,190
November	555	504	101	6	691	182	283	21	0	15	2,359
December	452	302	106	5	0	106	212	0	1	3	1,186
<b>Total</b>	<b>7,265</b>	<b>7,142</b>	<b>1,502</b>	<b>142</b>	<b>1,244</b>	<b>2,170</b>	<b>2,676</b>	<b>69</b>	<b>2</b>	<b>333</b>	<b>22,545</b>

Municipal Solid Waste quantities collected in tonnes for 2013

	January	February	March	April	May	June	July	August	September	October	November	December	Total
<b>Garbage</b>	553	492	501	655	768	780	868	887	653	672	556	585	7,970
<b>C &amp; D</b>	333	295	339	446	644	691	714	709	832	935	850	248	7,035
<b>Recycles</b>	140	94	109	126	110	129	168	137	121	135	104	94	1,468
<b>Scrap metals</b>	3	3	4	9	21	24	18	19	18	12	9	4	144
<b>Contaminated Waste</b>	0	3	6	41	398	0	9	58	9	1,507	0	0	2,030
<b>Sewere sludge</b>	211	204	241	192	286	132	237	194	204	268	130	149	2,447
<b>Ash</b>	238	305	262	298	282	180	214	245	199	254	200	258	2,934
<b>Electronics</b>	5	5	6	0	5	14	15	5	11	0	21	5	93
<b>Asbestos</b>	0	0	1	0	0	0	3	1	0	1	2	4	13
<b>Woodchips</b>	1	1	1	7	50	44	76	47	33	45	9	1	313
<b>Total</b>	<b>1,484</b>	<b>1,403</b>	<b>1,470</b>	<b>1,773</b>	<b>2,565</b>	<b>1,994</b>	<b>2,321</b>	<b>2,300</b>	<b>2,080</b>	<b>3,829</b>	<b>1,880</b>	<b>1,347</b>	<b>24,445</b>



Solid Waste Quantities 2012 in Tonnes

Month	Garbage	C& D	Recycles	Scrap Metals	Contaminated Waste	Sewarage sludge	Ash	Electronics	Asbestos	Wood chips	Total
January	508	277	110	8	149	173	157	7	0	0	1,390
February	436	323	122	7	136	221	256	0	0	1	1,501
March	570	692	134	10	30	188	245	5	0	11	1,885
April	554	737	88	17	103	161	250	0	0	32	1,943
May	669	618	135	15	15	238	239	6	0	50	1,985
June	682	725	130	13	9	228	184	6	0	66	2,043
July	793	656	149	18	0	143	169	0	0	41	1,969
August	841	626	162	17	19	183	173	18	1	43	2,084
September	561	791	127	12	93	138	243	6	0	39	2,009
October	644	891	137	15	0	208	264	0	0	30	2,190
November	555	504	101	6	691	182	283	21	0	15	2,359
December	452	302	106	5	0	106	212	0	1	3	1,186
<b>Total</b>	<b>7,265</b>	<b>7,142</b>	<b>1,502</b>	<b>142</b>	<b>1,244</b>	<b>2,170</b>	<b>2,676</b>	<b>69</b>	<b>2</b>	<b>333</b>	<b>22,545</b>



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**APPENDIX D**

**Ground Water Quality Data**

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# Ground Water Monitoring Data

Monitor	Date	QA/QC	As 0.025 mg/L	B 5 mg/L	Ba 1 mg/L	Ca mg/L	Cd 0.005 mg/L	Cr 0.05 mg/L	Cu 1 mg/L	Fe 0.3 mg/L	Hg 0.001 mg/L	K mg/L	Mg mg/L	Mn 0.05 mg/L
ODWQS - Units -														
11/4	6/1/2001			0.012	0.023	10.7				0.10			3.5	
	8/1/2001			< 0.001	0.035	11.7				<b>0.43</b>			3.6	
	5/7/2002			< 0.001	0.025	11.1				0.13			3.1	
	8/6/2002			< 0.001	0.049	14.6				<b>1.02</b>			4.6	
	5/26/2004			<0.01	0.022	9.3				0.04			2.9	
	8/24/2004			0.01	0.023	11.9				0.03			3.2	
	8/3/2005			< 0.01	0.025	12.0				< 0.05			3.5	
	9/1/2005			< 0.01	0.026	12.0				< 0.05			3.7	
	8/14/2007			< 0.02	< 0.02	8.2				< 0.05			2.3	
	5/21/2008			< 0.02	< 0.02	7.7				< 0.05			2.2	
	8/19/2008			< 0.02	< 0.02	7.7				< 0.05			2.3	
	5/19/2009			0.09	1.010	8.0				0.52			2.4	
	8/18/2009			< 0.02	< 0.02	8.3				< 0.05			2.4	
	5/18/2010			<0.02	0.02	8.7				0.06			2.5	
	8/24/2010			0.18	0.850	7.6				0.03			2.3	
	5/31/2011			<0.02	<0.02	8.3				<0.05			2.5	
	8/23/2011			<0.02	<0.02	9.0				<0.02			2.5	
5/16/2012	Dry													
8/29/2012				<0.02	<0.02	9.3				0.03			2.6	
5/22/2013	Dry													
8/27/2013	Dry													
5/20/2014				<0.02	0.021	8.8				0.02			2.5	
8/19/2014			<0.001	<0.01	0.018	6.9	<0.001	<0.005	0.0014	<0.1	<0.0001	1.1	2.0	0.0024
Monitor	Date	QA/QC	Na 200 mg/L	Pb 0.01 mg/L	Zn 5 mg/L	Cl- 250 mg/L	NO2-N mg/L	NO3-N 10 mg/L	SO4 500 mg/L	pH 6.5 - 8.5	alk42 500 mg/L	NH3-N mg/L	DOC 5.0 mg/L	Th Cond µS/cm
ODWQS - Units -														
11/4	6/1/2001		5.9			1.2		0.10	3.5	7.89	46	< 0.10	4.9	78
	8/1/2001		6.5					0.30		6.99	30	< 0.10	2.7	115
	5/7/2002		5.4			7.3	< 0.10	3.3	6.94	50	0.04	3.0	109	
	8/6/2002		7.4			11.0	< 0.03	4.0	7.30	46	0.03	2.4	98.2	
	5/21/2003	Frozen												
	8/24/2004		5.9			0.9		0.30	4.7	6.66	42	0.09	3.9	101
	8/3/2005		9.8			< 1		1.30	7.0	7.19	51	< 0.05	<b>5.3</b>	104
	9/1/2005		9.2			< 1		0.30	10.4	7.10	53	0.05	<b>5.9</b>	103
	5/16/2006	Dry												
	5/21/2008		5.3			< 1	< 0.1	5	7.20	36	< 0.1	4.4	82	
	8/19/2008		5.8			< 1	< 0.1	4	7.30	39	< 0.1	<b>6.2</b>	84	
	5/19/2009		9.1			< 1	< 0.1	4	6.60	38	0.06	3.5	93	
	8/18/2009		5.9			< 1	0.1	4	6.60	44	0.07	4.2	91	
	5/18/2010		6			< 1	< 0.1	4	7.00	43	0.12	3.7	96	
	8/24/2010		11.3			< 1	0.1	4	6.80	43	< 0.05	4.2	92	
	5/31/2011		5.6			< 1	< 0.1	3	6.99	41	0.1	3.9	92	
	8/23/2011		6.1			< 1	< 0.1	4	6.73	42	< 0.05	3.9	92	
5/16/2012	Dry													
8/29/2012		5.8			9		0.14	4	6.92	47	NA	4.6	100	
5/22/2013	Dry													
8/27/2013	Dry													
5/20/2014		6.3			< 1	< 0.1	4	6.74	38	< 0.05	4.0	84		
8/19/2014		5.3	< 0.0005	< 0.005	9	< 0.01	< 0.1	4	6.79	35	0.066	4.0	83	
Monitor	Date	QA/QC	Th TDS 500 mg/L	TKN mg/L	Total P mg/L	COD mg/L	BOD mg/L	TSS mg/L	Phenols mg/L	Field pH 6.5 - 8.5 mg/L	Field Cond µS/cm			
ODWQS - Units -														
11/4	6/1/2001		136			36	7.9							
	8/1/2001		219			33								
	5/7/2002		184			84	3.0	3.74		7.00	60			
	8/6/2002		152			28	< 1	0.45		6.70	90			
	5/21/2003	Frozen												
	8/24/2004		176			13				NA	NA			
	8/3/2005		175			< 4	< 2	7700		<b>5.40</b>	70			
	9/1/2005		<b>550</b>			14				<b>4.80</b>	90			
	5/16/2006	Dry												
	5/21/2008		56			< 4	< 2	27000		7.54				
	8/19/2008		60			15	< 2	14000		7.68				
	5/19/2009		60			28				6.83	100			
	8/18/2009		57			53				6.63	88			
	5/18/2010		64			83				7.52	91			
	8/24/2010		58			26				<b>11.15</b>				
	5/31/2011		54			9				<b>6.26</b>	100			
	8/23/2011		74			11				6.51	93			
5/16/2012	Dry													
8/29/2012		NA			NA				<b>6.30</b>	136				
5/22/2013	Dry													
8/27/2013	Dry								6.99	112				
5/20/2014		<b>1270</b>			21				7.56	78				
8/19/2014		<b>562</b>	< 1	25	11	< 2		< 0.001	7.24	105				

Notes: **Bold** denotes exceedance of ODWQS (2006) criteria

NA - Not Analyzed

















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**APPENDIX E**

**Ground Water Levels**

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## Historical Water Level Monitoring Data

Monitor	Target Zone	Surface Elevation (masl)	Trigger Elevation (masl)	Background Fenco (10/96)	Ground Water Elevations										
					1-May-02	1-Jun-02	1-Jul-02	1-Aug-02	1-Sep-02	1-Oct-02	1-May-03	1-May-04	1-Aug-04	1-Aug-05	1-Sep-05
1/17	Till	351.44		350.87	350.66			350.23			350.74	350.74	NA	NA	NA
2/9	Till	351.07		350.08	350.32	350.26	350.17	350.16	350.32	349.62	349.62	350.62	350.36	350.23	350.32
2/13	Bedrock	350.85		350.06	350.10			349.93			350.18	350.35	350.15	350.13	350.13
3/8	Till	351.51		350.44	349.86	350.17	350.06	350.06	350.36	350.30	350.34	350.51	350.21	350.19	350.13
4/6	Till	351.48		350.36	350.13			349.93			349.79	350.18	350.08	350.06	350.00
5/17	Till / Bedrock	370.13	361.75	355.57	353.08	352.82	352.63	N/A	352.43	352.58	353.59	353.83	357.10	357.77	357.18
6/5	Till	351.09		349.90	349.77			349.55			349.71	349.97	349.87	349.79	349.75
11/4	Till	366.45	366.45		361.36	361.34	361.41	N/A	361.34	362.11	361.95	363.85	363.19	363.41	363.01
13/6	Till	355.02			348.62	348.90	348.90	348.81	349.09	349.58		350.37	350.26	350.22	349.90
13/14	??	354.72			349.27	350.00	349.15	348.72	349.62	349.87		351.22	350.63	350.78	350.27
14/6	Till	348.91			346.07	346.50	346.48	346.33	346.91	347.66		348.19	347.92	347.81	347.57
14/21	??	348.68			345.18	345.50	345.45	345.30	345.92	346.53		347.18	346.91	346.72	346.32
15/5	Till	352.17			350.50	349.80	350.37	350.38	349.80	350.88		351.42	350.73	350.69	350.51
15/17	??	352.08			350.58	350.60	350.47	350.47	350.85	350.83		351.99	350.80	358.59	358.57
16/15	??	362.25	362.25		350.37	350.07	349.95	349.94	Dry	350.27	350.51	350.55	351.04	352.05	352.13
17/15	??	359.99			350.37			350.14			350.72	350.99	351.79	352.47	352.09
19/6	Till	364.15	364.15		357.60	357.54	364.15	N/A	Dry	Dry	357.54	357.45	dry	dry	353.02
23/3	Peat	351.19			350.33			350.12			350.46	350.39	351.00	350.27	350.25
24/5	Till	352.01			350.56			350.80			350.89	351.01	350.91	350.79	350.79
KGS-2	??	351.38		350.37	349.93			349.66			350.08	350.08	350.08	350.20	350.20

Note: ?? denotes unknown target zone

## Historical Water Level Monitoring Data

Monitor	Target Zone	Surface Elevation (masl)	Trigger Elevation (masl)	Background Fenco (10/96)	Ground Water Elevations											
					1-Oct-05	1-May-06	1-Aug-06	1-May-07	1-Aug-07	1-May-08	1-Aug-08	1-May-09	1-Aug-09	10-May-10	10-Aug-10	9-Jun-11
1/17	Till	351.44		350.87	NA	350.66	350.04	350.38	351.44	349.37	no access	no access	349.99	349.47	350.00	349.50
2/9	Till	351.07		350.08	349.62	349.95	349.77	349.82	349.85	349.88	349.79	349.86	349.84	349.74	349.74	349.72
2/13	Bedrock	350.85		350.06	350.14	350.10	349.93	350.06	350.02	350.14	350.07	350.12	350.15	350.08	350.18	350.16
3/8	Till	351.51		350.44	350.16	350.17	349.80	350.12	350.26	350.28	350.11	350.38	350.31	350.22	350.26	350.25
4/6	Till	351.48		350.36	350.39	350.05	349.73	350.13	350.16	350.29	350.23	350.85	350.58	350.52	350.54	350.52
5/17	Till / Bedrock	370.13	361.75	355.57	356.10	dry	dry	Dry	354.91	353.54	354.88	353.28	356.27	353.65	356.05	353.58
6/5	Till	351.09		349.90	349.79	349.73	349.02	349.65	349.75	349.81	349.55	349.87	349.84	349.74	349.84	349.80
11/4	Till	366.45	366.45		362.05	dry	dry	Dry	362.75	362.26	362.82	362.61	363.45	362.22	363.57	362.53
13/6	Till	355.02			349.52	349.41	348.67	348.56	350.50	349.50	350.10	349.77	350.62	349.48	350.57	349.63
13/14	??	354.72			350.22	350.07	348.39	349.09	350.35	349.69	350.16	350.02	350.77	350.22	350.76	350.26
14/6	Till	348.91			347.41	347.31	346.32	345.05	347.32	346.86	347.27	347.50	348.08	347.41	347.91	347.61
14/21	??	348.68			346.37	346.33	345.33	344.20	346.30	345.91	346.26	346.55	347.03	346.43	347.86	347.56
15/5	Till	352.17			350.52	350.57	350.12	349.45	350.87	350.77	350.58	351.07	350.97	350.71	350.88	350.73
15/17	??	352.08			359.53	350.66	350.26	350.56	350.82	350.78	350.67	350.95	350.92	350.73	350.90	350.75
16/15	??	362.25	362.25		351.56	350.50	350.19	349.60	350.63	350.44	350.91	350.46	351.41	350.62	356.58	350.70
17/15	??	359.99			351.53	350.58	350.23	349.72	351.40	350.55	350.99	350.62	351.64	350.59	351.84	350.83
19/6	Till	364.15	364.15		dry	dry	dry	Dry	Dry	Dry	Dry	Dry	Dry	dry	357.56	dry
23/3	Peat	351.19			350.30	350.22	349.85	350.17	350.32	350.39	350.32	350.44	350.63	349.79	350.48	350.57
24/5	Till	352.01			351.01	350.79	350.53	350.52	350.75	350.83	350.89	341.74	351.04	350.61	351.01	351.01
KGS-2	??	351.38		350.37	349.34	349.98	349.79	349.84	no access	348.87	no access	351.38	no access	348.45	349.08	349.97

Note: ?? denotes unknown target zone

## Historical Water Level Monitoring Data

Monitor	Target Zone	Surface Elevation (masl)	Trigger Elevation (masl)	Background Fenco (10/96)	Ground Water Elevations							Maximum (masl)	Minimum (masl)	Fluctuation (m)
					23-Aug-11	16-May-12	29-Aug-12	21-May-13	26-Aug-13	20-May-14	19-Aug-14			
1/17	Till	351.44		350.87	351.44	349.02	349.22	349.02	349.22	348.00	348.19	351.44	348.00	<b>3.44</b>
2/9	Till	351.07		350.08	349.67	349.57	349.57	349.57	349.57	349.77	349.60	350.62	349.57	<b>1.05</b>
2/13	Bedrock	350.85		350.06	350.07	350.07	350.06	350.07	350.06	350.34	350.17	350.35	349.93	<b>0.42</b>
3/8	Till	351.51		350.44	350.13	350.09	350.08	350.09	350.08	350.45	350.19	350.51	349.80	<b>0.71</b>
4/6	Till	351.48		350.36	N/A	N/A	N/A	NA	350.08	349.90	348.94	350.85	348.94	<b>1.91</b>
5/17	Till / Bedrock	370.13	361.75	355.57	354.68	352.43	352.73	352.43	352.73	352.44	355.79	357.77	352.43	<b>5.34</b>
6/5	Till	351.09		349.90	349.48	349.62	349.30	349.62	349.30	349.93	349.67	349.97	349.02	<b>0.95</b>
11/4	Till	366.45	366.45		362.58	361.24	361.59	361.24	361.59	361.95	362.93	363.85	361.24	<b>2.61</b>
13/6	Till	355.02			349.62	348.73	349.33	348.73	349.33	349.57	350.07	350.62	348.56	<b>2.06</b>
13/14	??	354.72			349.63	348.96	349.28	348.96	349.28	349.88	350.11	351.22	348.39	<b>2.83</b>
14/6	Till	348.91			346.98	346.11	346.46	346.11	346.46	346.99	347.12	348.19	345.05	<b>3.14</b>
14/21	??	348.68			346.00	345.16	345.49	345.16	345.49	346.10	346.07	347.86	344.20	<b>3.66</b>
15/5	Till	352.17			350.56	350.62	350.53	350.62	350.53	351.34	350.68	351.42	349.45	<b>1.97</b>
15/17	??	352.08			350.66	350.53	350.59	350.53	350.59	351.11	349.75	359.53	349.75	<b>9.78</b>
16/15	??	362.25	362.25		350.93	349.71	350.41	349.71	350.41	350.35	350.95	356.58	349.60	<b>6.98</b>
17/15	??	359.99			351.05	350.04	350.29	350.04	350.29	350.47	351.52	352.47	349.72	<b>2.75</b>
19/6	Till	364.15	364.15		dry	dry	dry	350.31	350.75	350.57	351.84	364.15	350.31	<b>13.84</b>
23/3	Peat	351.19			350.50	350.46	350.45	350.46	350.45	350.14	349.28	351.00	349.28	<b>1.72</b>
24/5	Till	352.01			350.79	350.69	350.84	350.69	350.84	350.40	349.65	351.04	341.74	<b>9.30</b>
KGS-2	??	351.38		350.37	348.95	348.74	348.83	348.74	348.83	347.91	347.56	351.38	347.56	<b>3.82</b>

Note: ?? denotes unknown target zone



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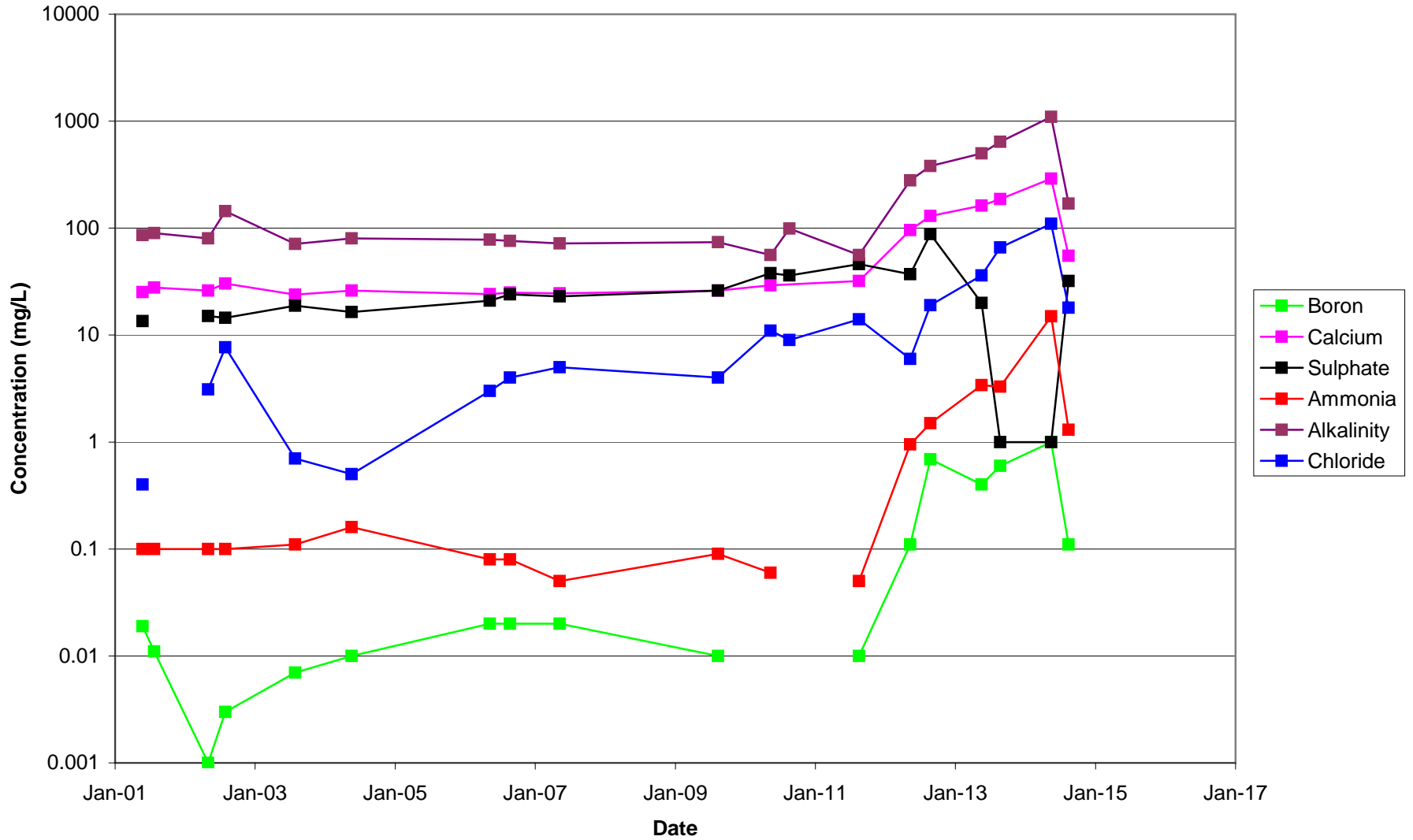
## **APPENDIX F**

### **Ground Water Chemistry Over Time**

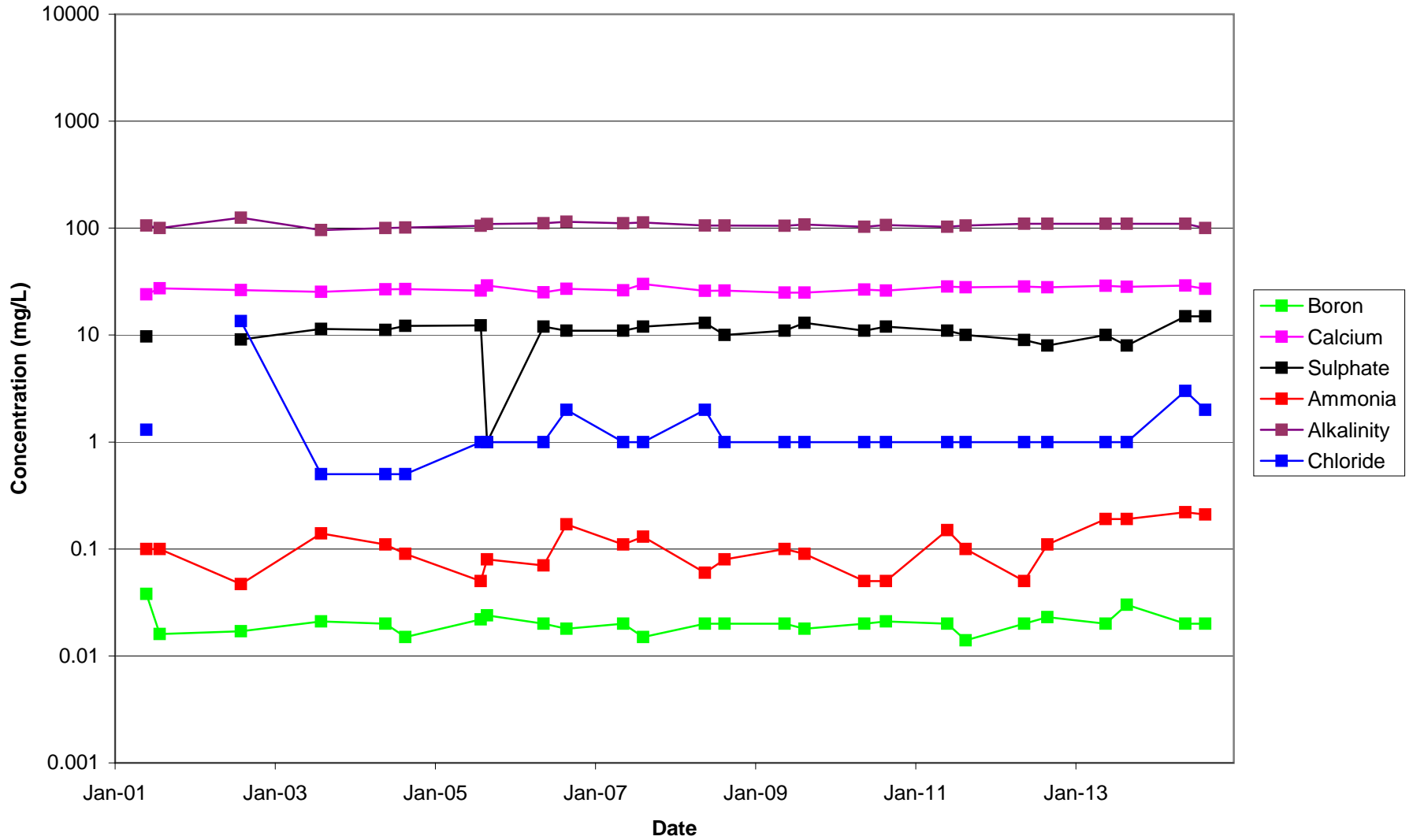
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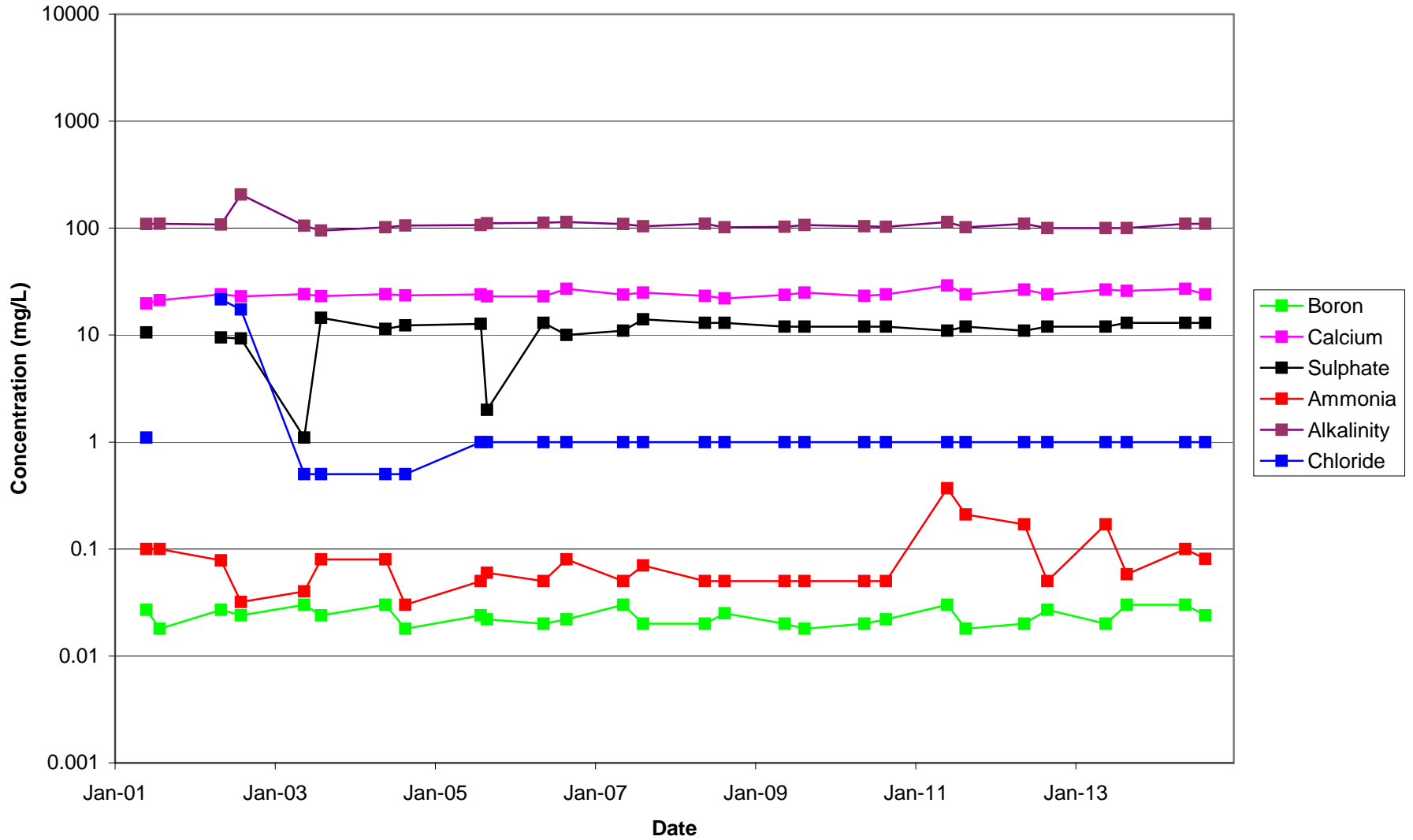
# MW1/17



# MW2/9

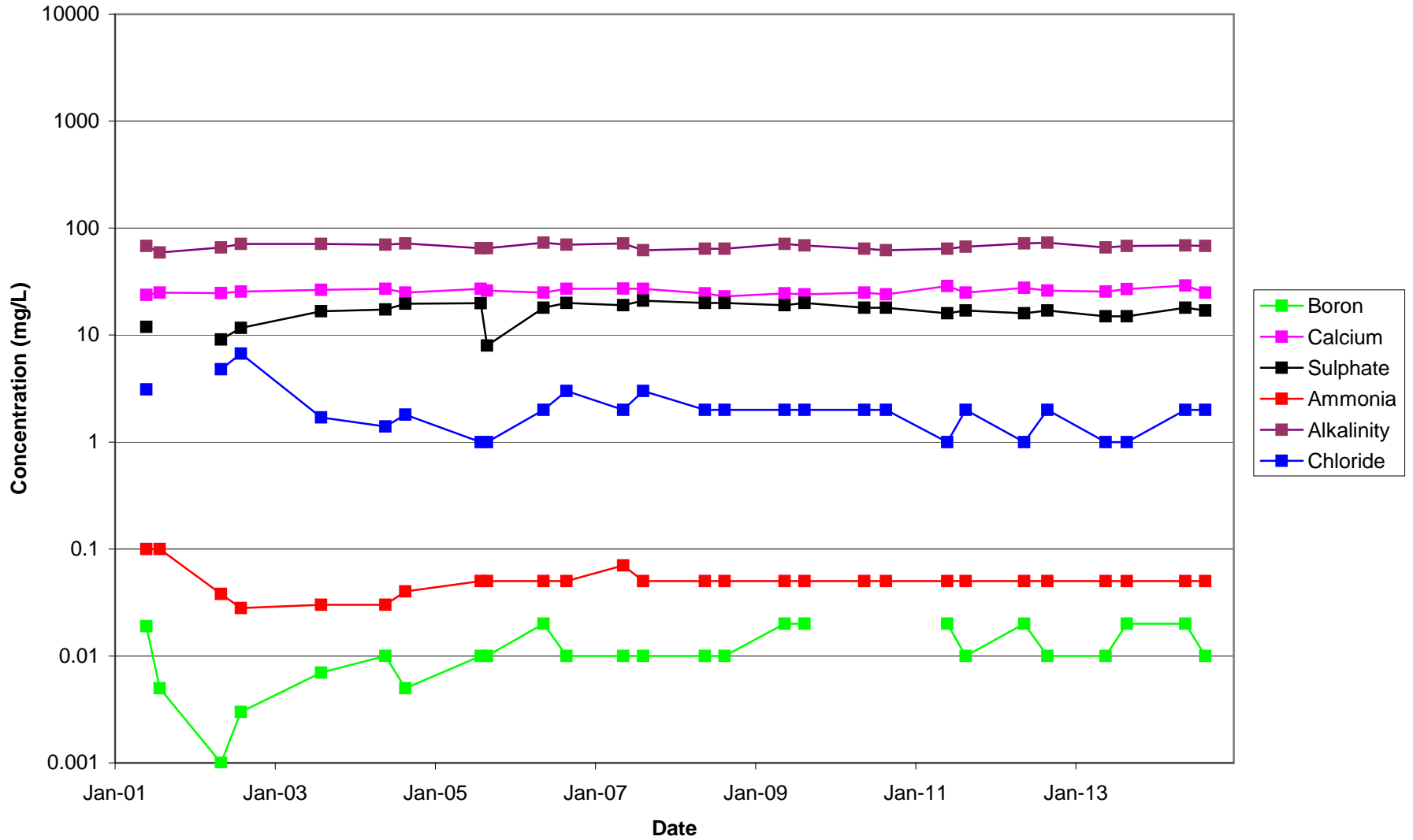


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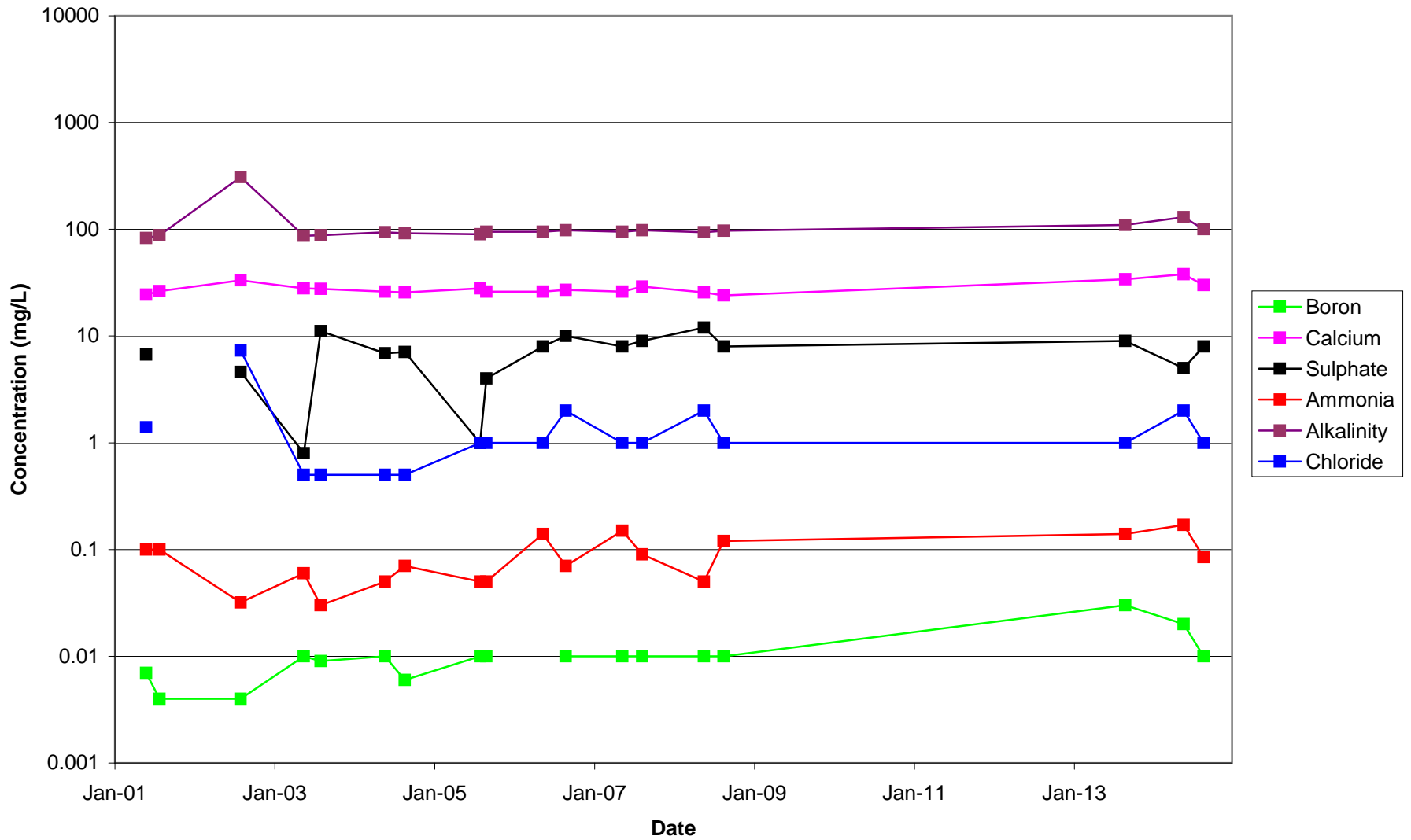




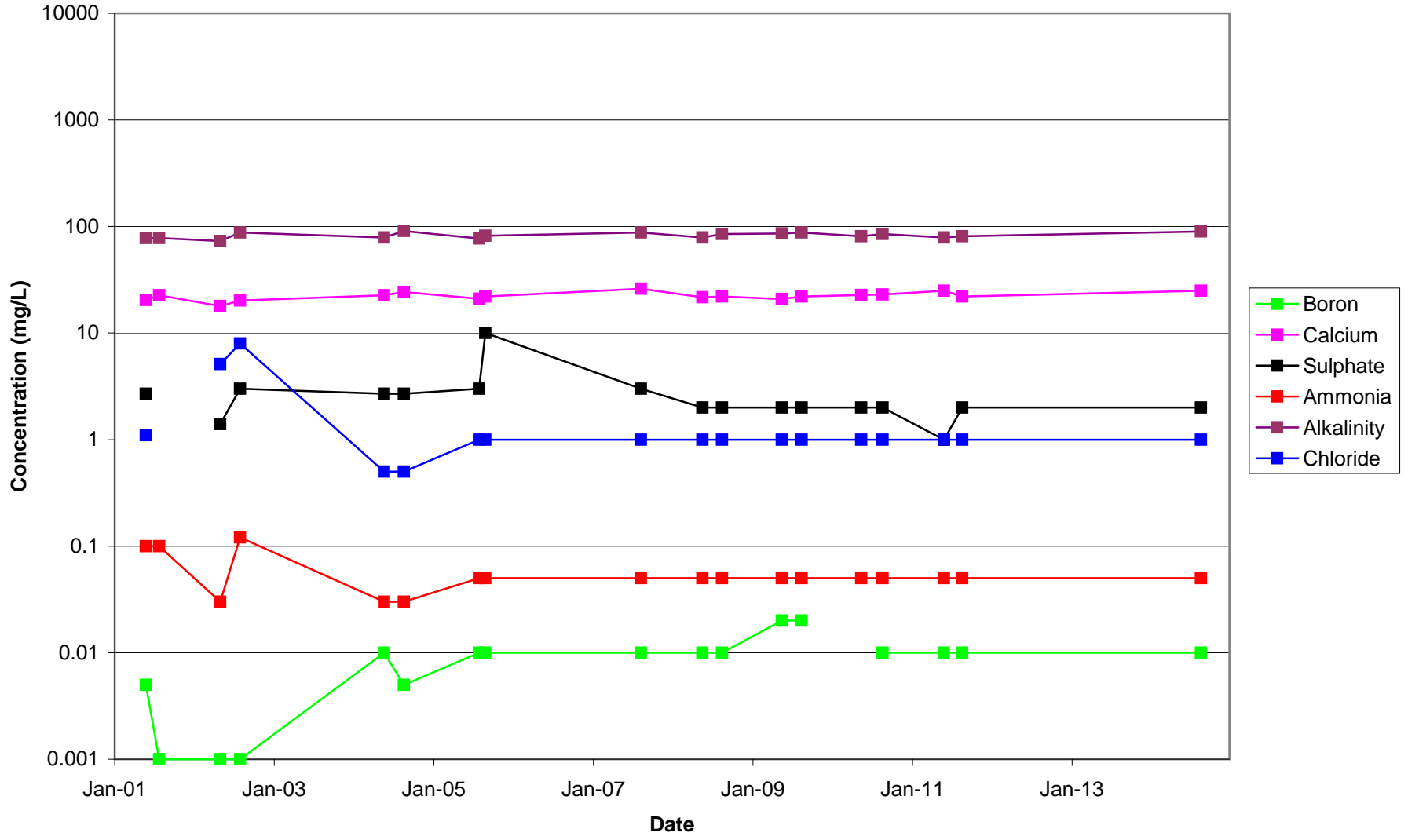
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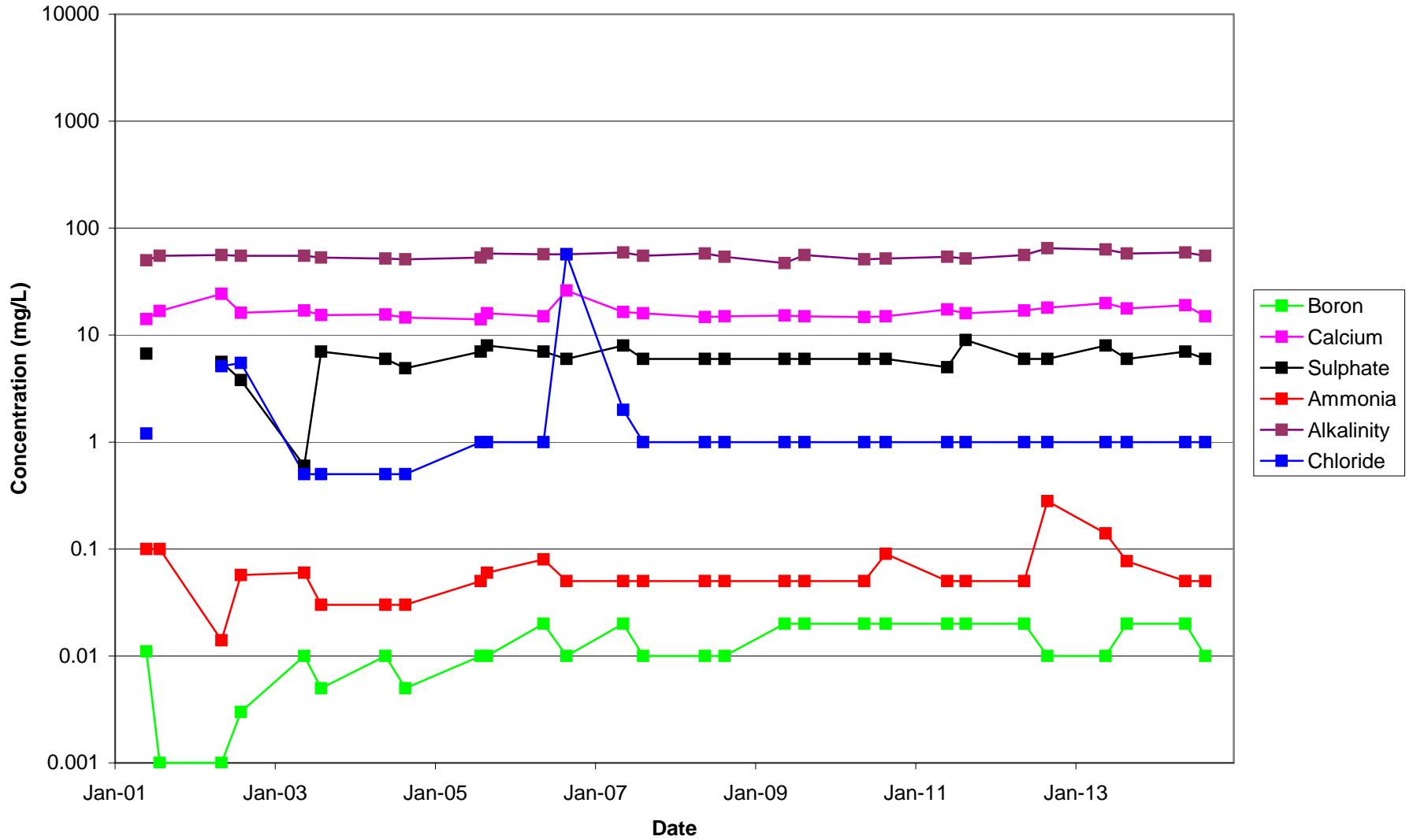
# MW4/6



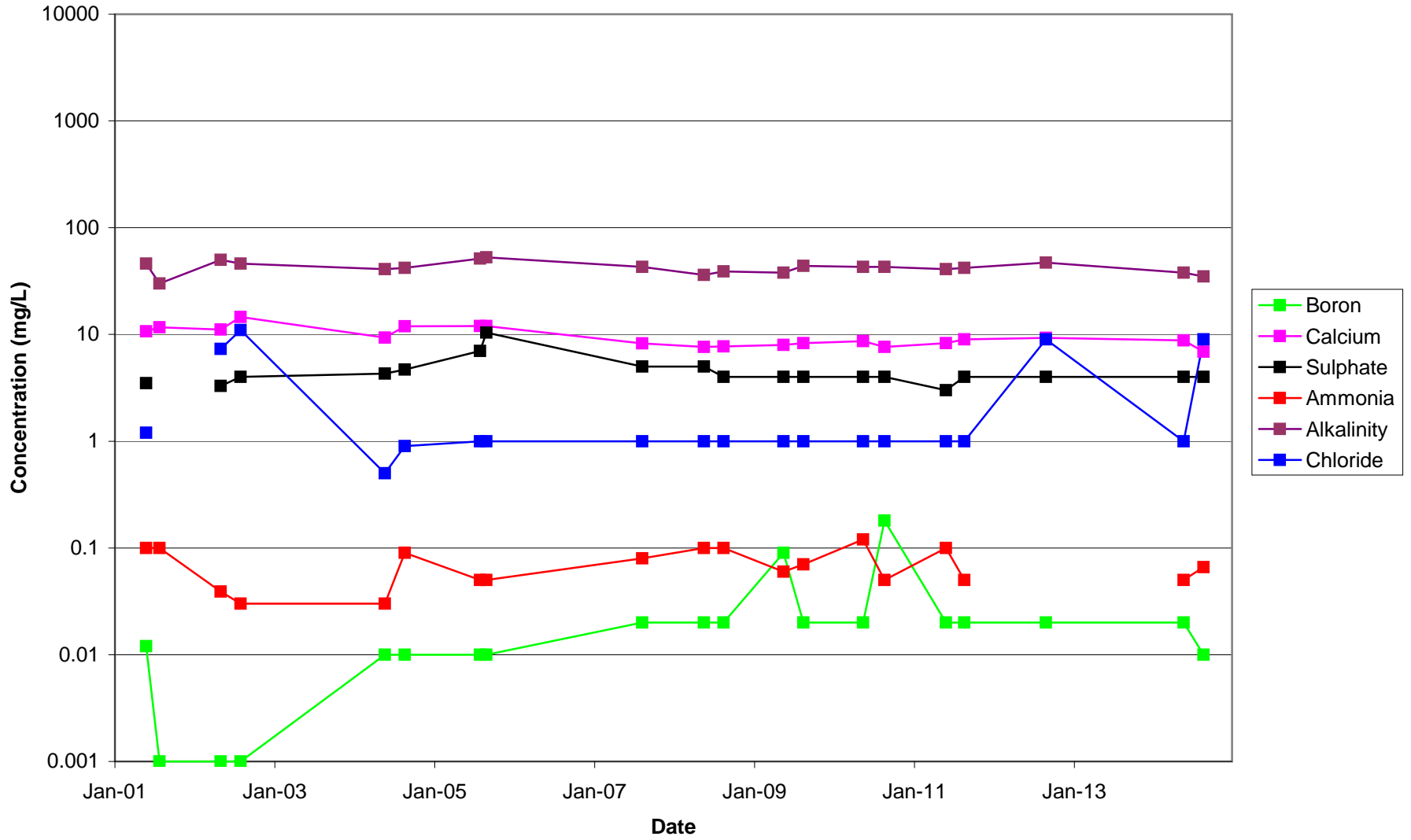
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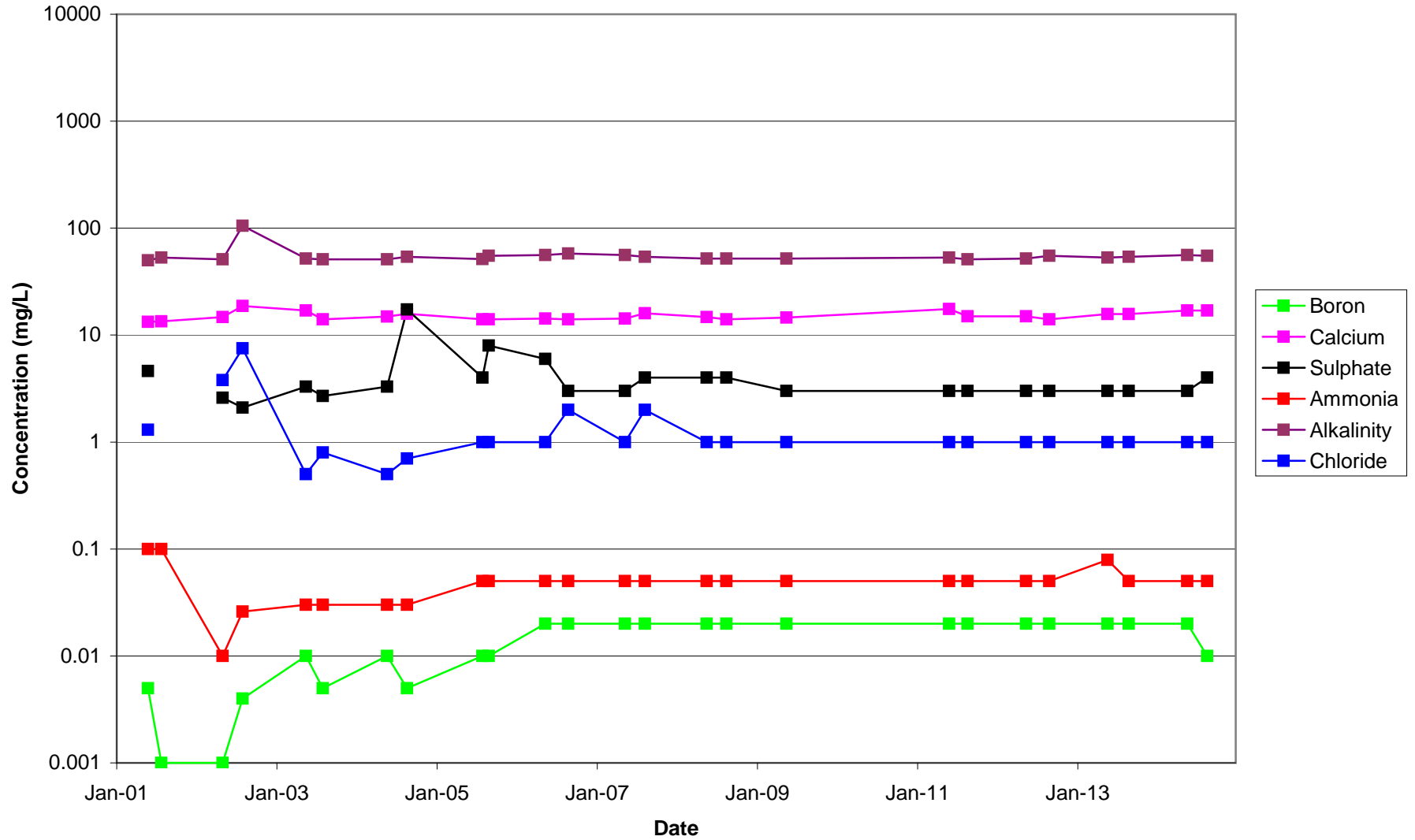
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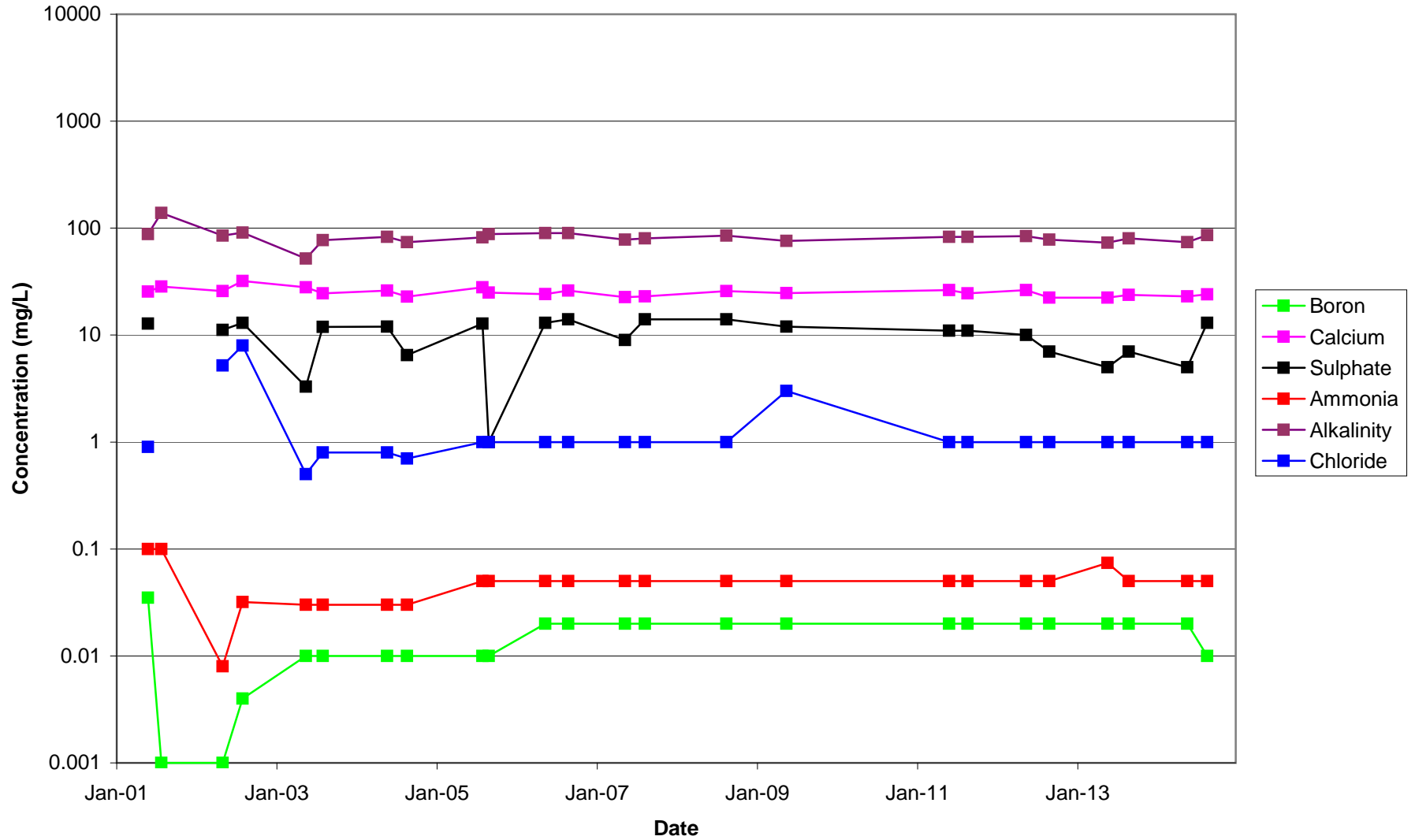
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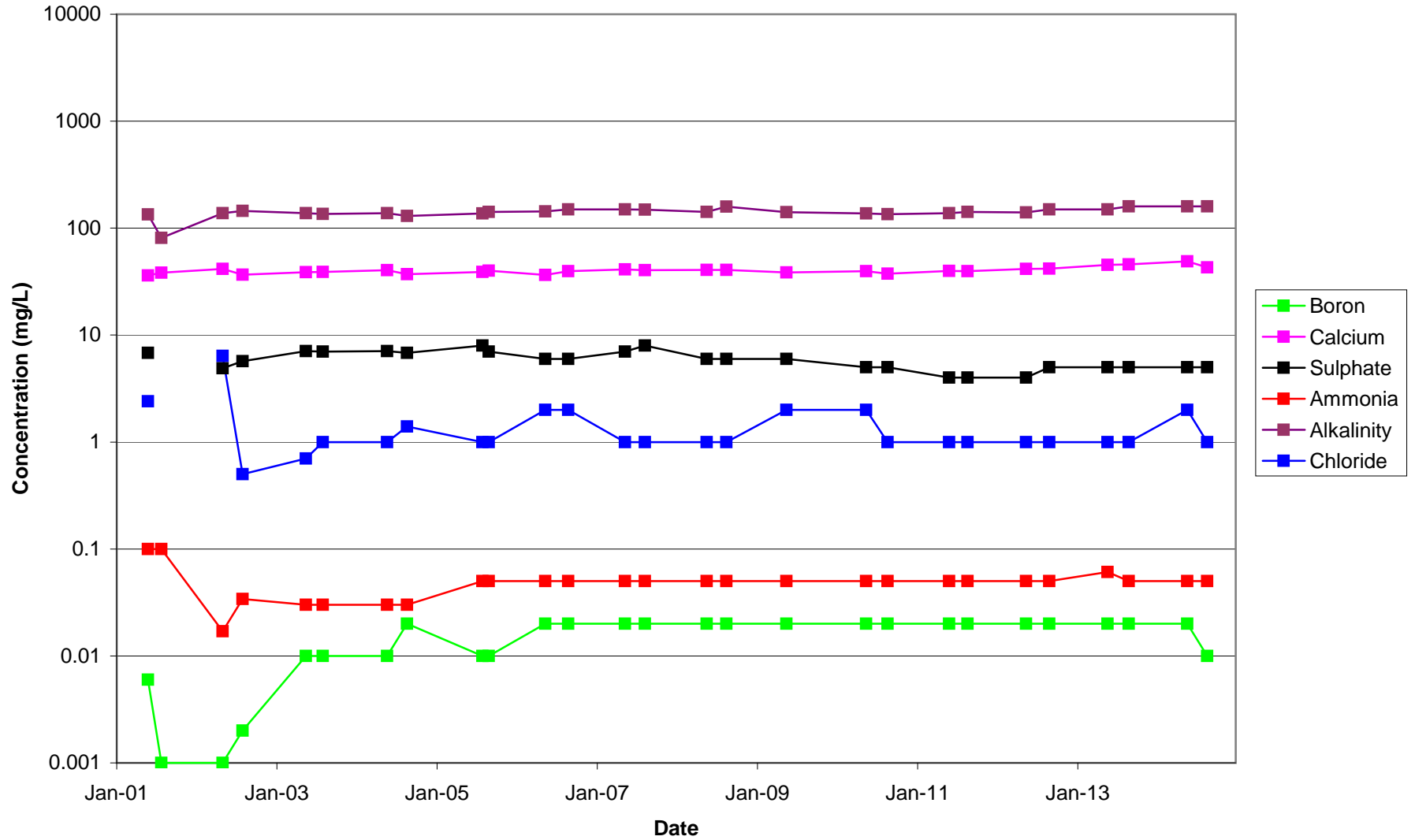
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# MW17/15

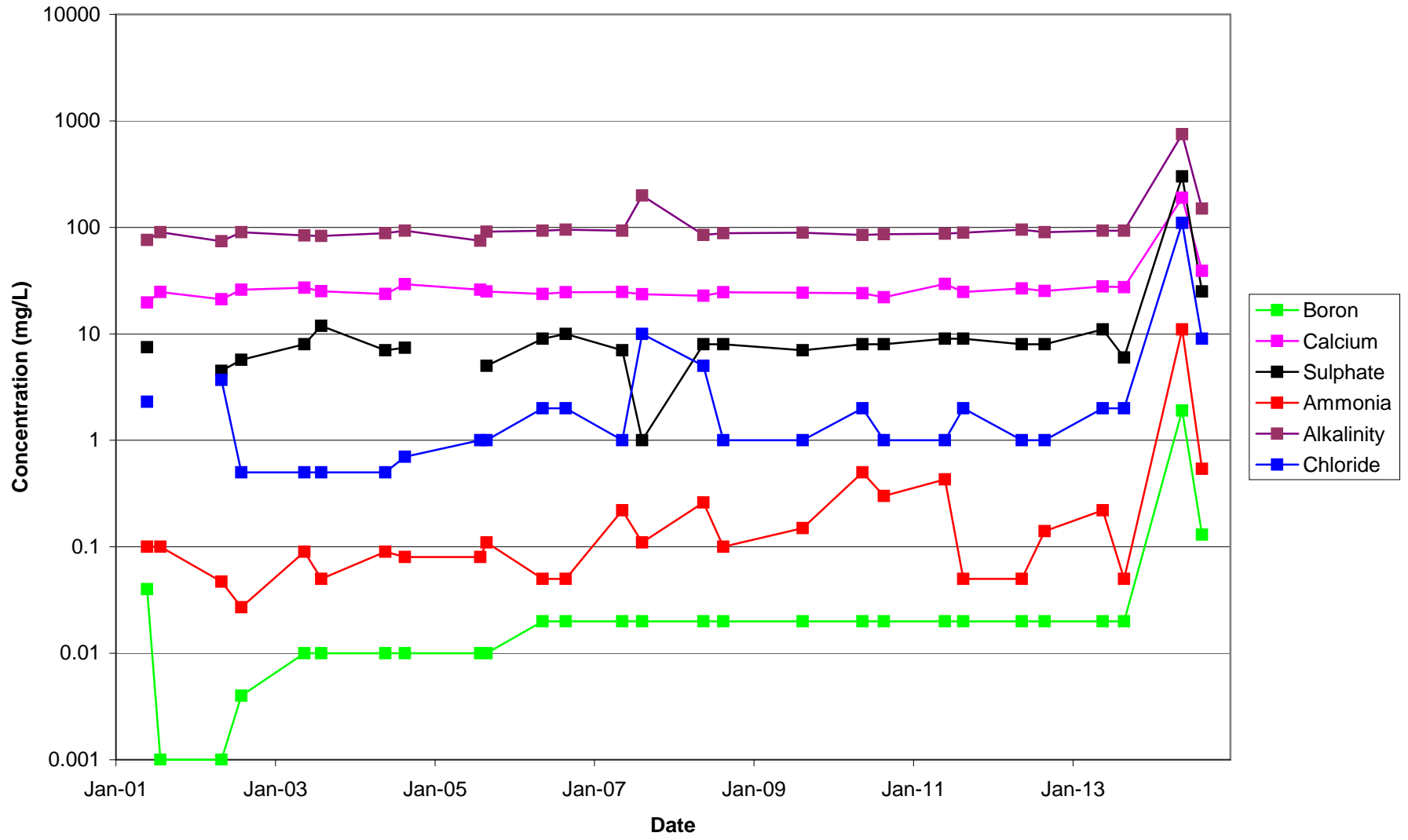


# MW19/16

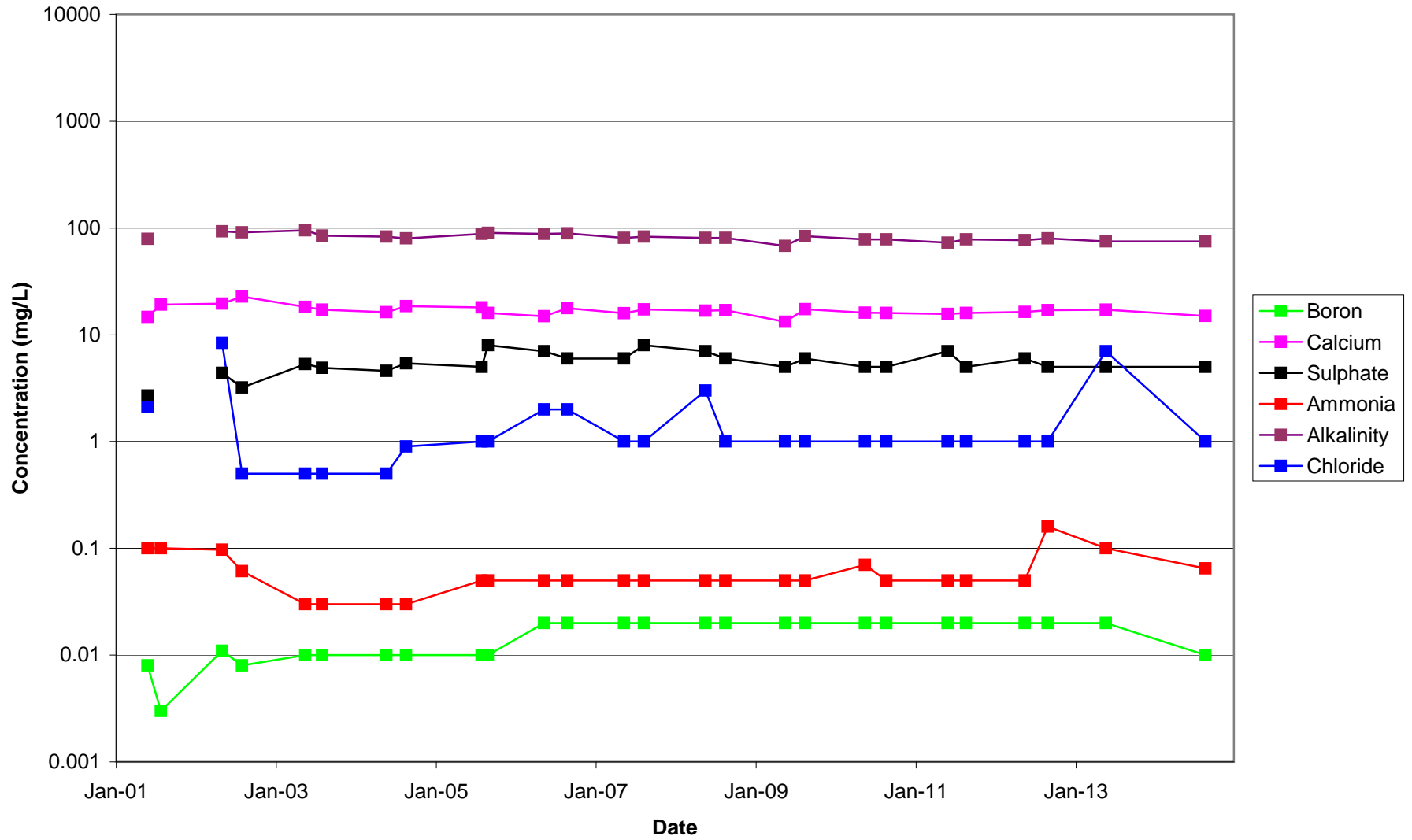




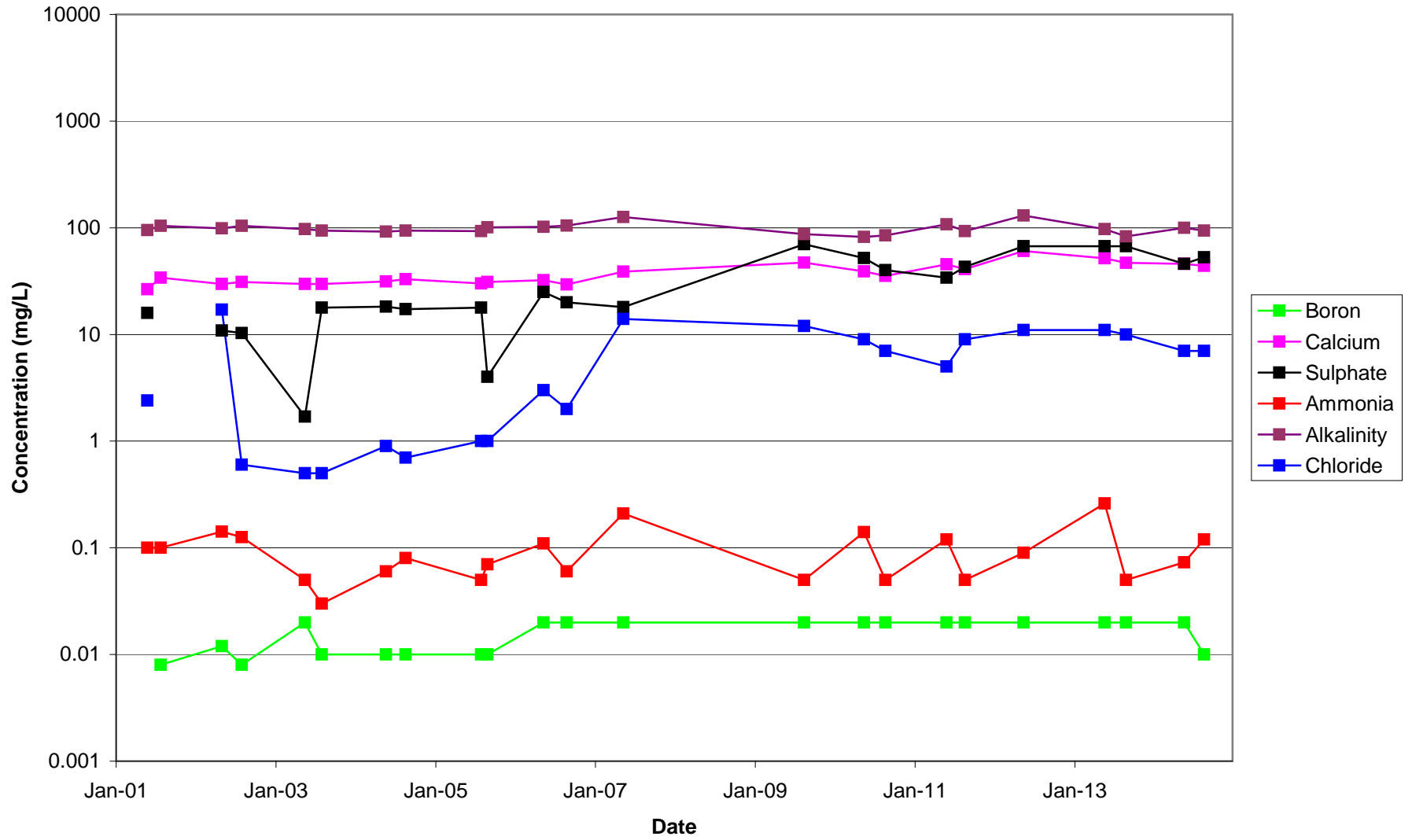
# MW23/3



# MW24/5



# KGS-2





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## **APPENDIX G**

### **Detected Volatile Organic Compounds**

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## Summary Table of Detected Volatile Organic Compounds

Monitor	Date	Toluene (mg/L) <i>0.024 (ODWQS, 2006)</i>	1,4 dichloro-benzene (mg/L) <i>0.001 (ODWQS, 2006)</i>
4/18	August, 2001	-	0.001
1/17	August, 2002	0.001	-
2/9	August, 2002	0.0006	-
3/8	August, 2002	0.0026	-
4/6	August, 2002	0.0013	-
4/18	August, 2002	0.003	-
5/17	August, 2002	0.0005	-
6/5	August, 2002	0.0021	-
20/4	August, 2004	0.0002	-
4/6-II	August, 2008	0.0004	-
SW-2	August, 2010	0.001	-

All results for all other required sampling parameters were below laboratory detection limits.

In 2006, VOC samples were collected at monitors: 1/17, 2/9, 2/13, 3/8, 4/6, 4/18, 6/14, 7/4, 16/15, 20/4, 21/7 and SW-2

In 2007, VOC samples were collected at monitors: 2/9, 2/13, 3/8, 4/6, 4/18, 5/17, 6/5, 6/5 II, 6/14, 7/4, 16/15, 20/4, 21/7 and SW-2

In 2008, VOC samples were collected at monitors: 2/9, 2/13, 3/8, 4/6, 4/6-II, 4/18, 5/17, 6/5, 6/14, 7/4, 16/15, 20/4, 21/7 and SW-2

In 2009, VOC samples were collected at monitors: 1/17, 2/9, 2/13, 3/8, 5/17, 6/5, 6/14, 7/4, 20/4, 21/7 and SW-2

In 2010, VOC samples were collected at monitors: 2/9, 2/13, 3/8, 5/17, 6/5, 6/14, 7/4 and SW-2

In 2011, VOC samples were collected at monitors: 2/9, 2/13, 3/8, 5/17, 6/5, 6/14, 7/4, 16/15, 21/7 and SW-2

In 2012, VOC samples were collected at monitors: 2/9, 2/13, 3/8, 5/17, 6/5, 6/14, 7/4, 16/15, 21/7 and SW-2

In 2013, VOC samples were collected at monitors: 2/9 and SW-2

In 2014, VOC samples were collected at monitors: 2/9 and SW-2



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**APPENDIX H**

**Surface Water Quality Data**

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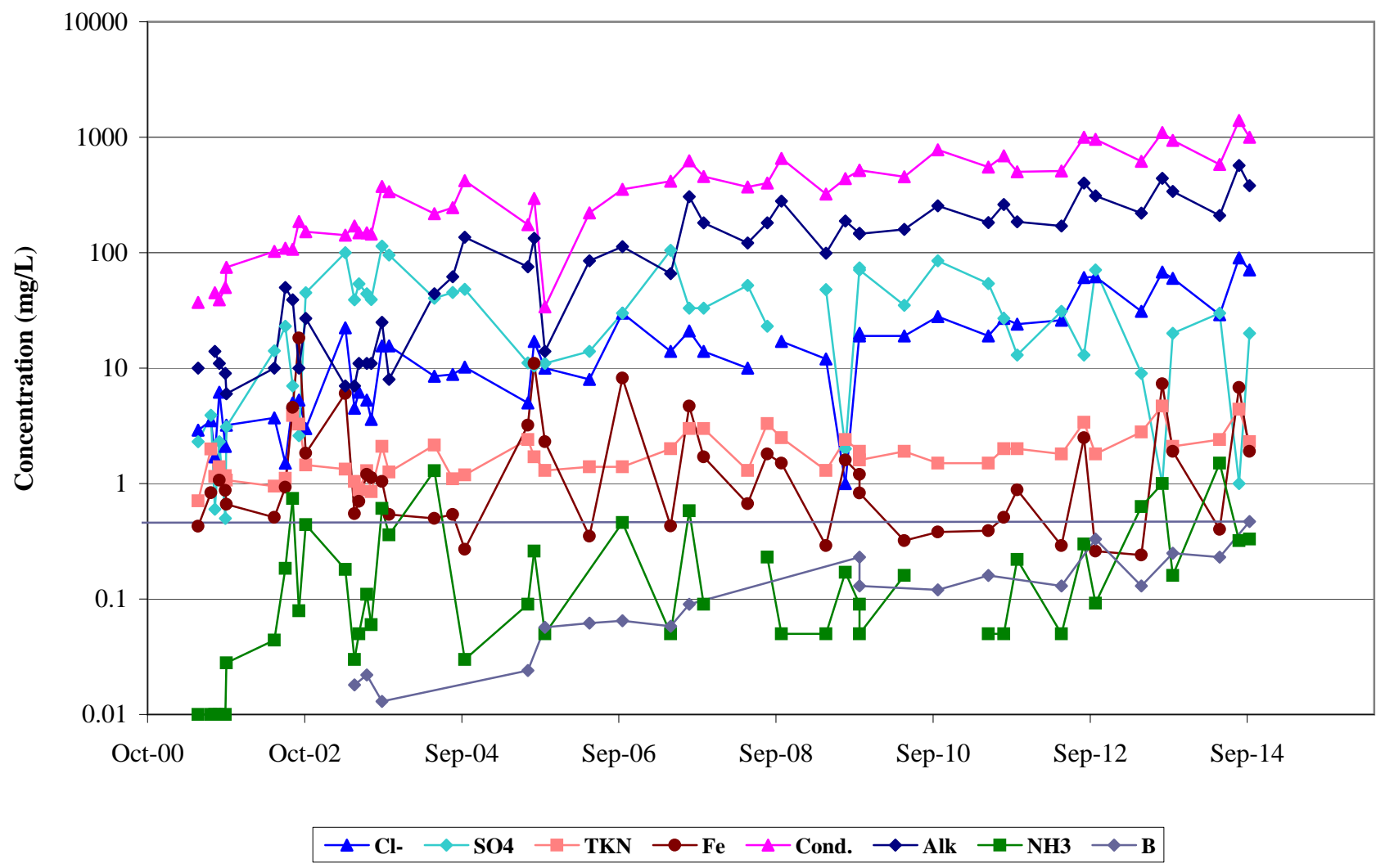
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## **APPENDIX I**

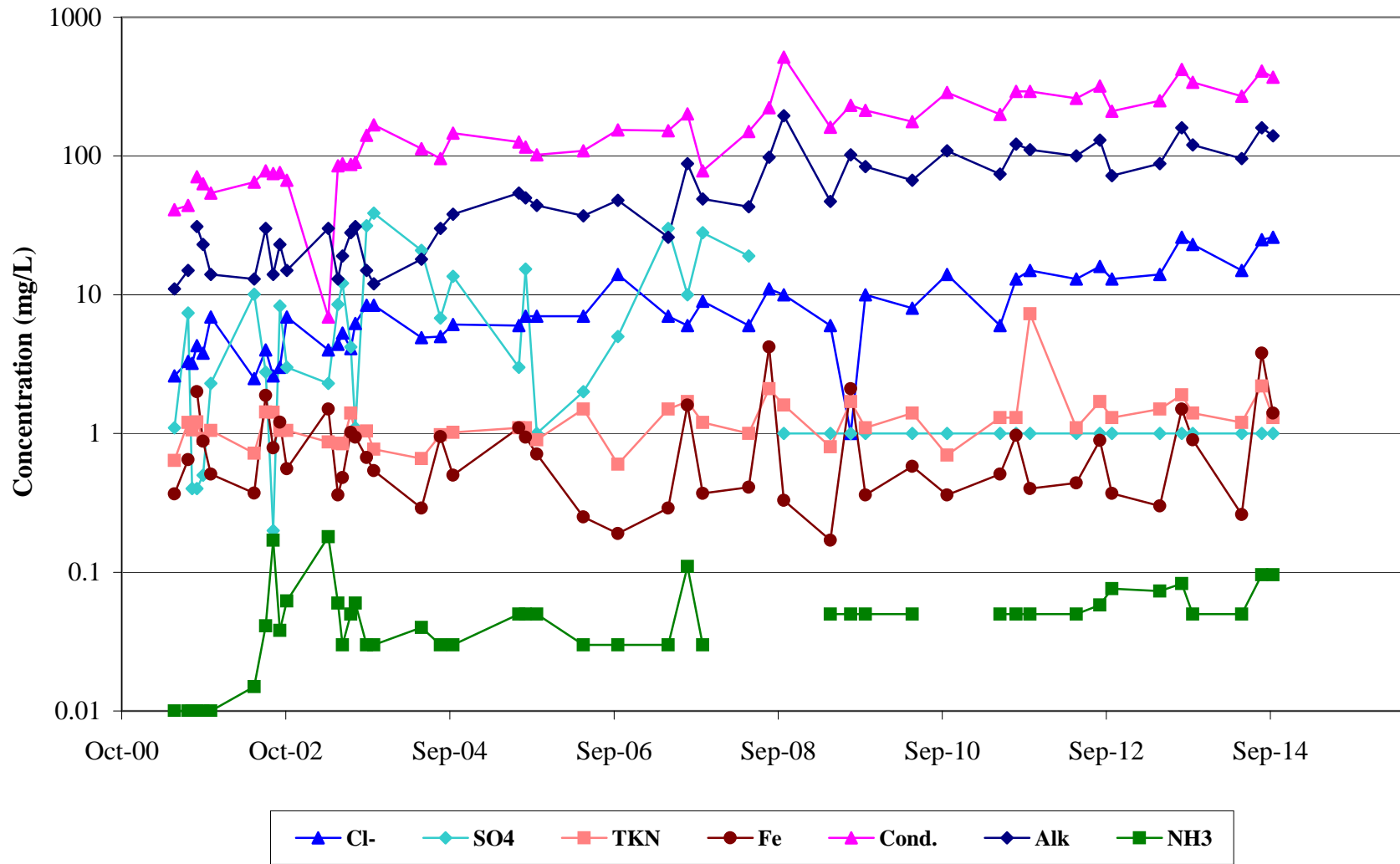
### **Surface Water Quality Over Time**

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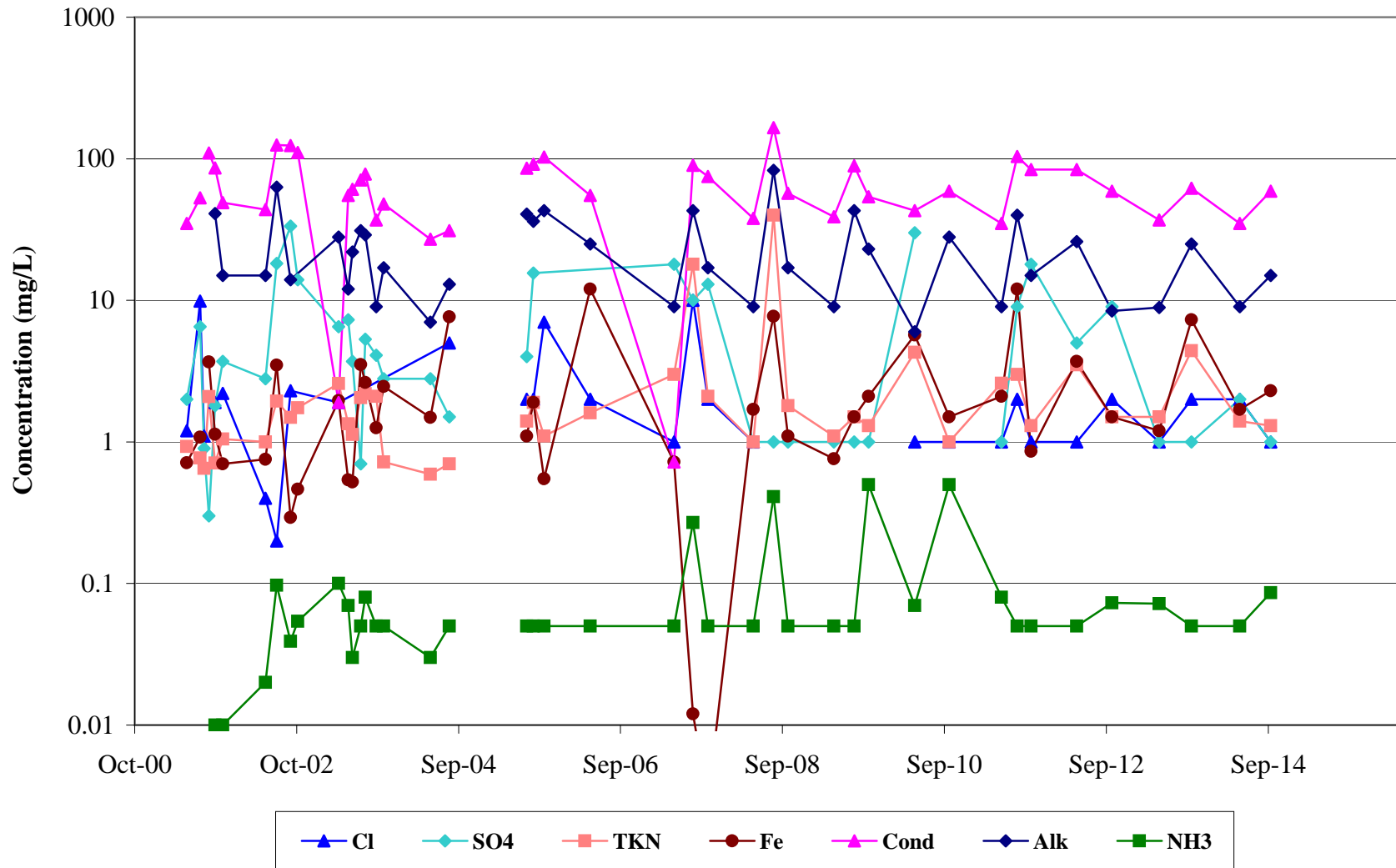
# Chemistry Over Time At SW-1



# Chemistry Over Time At SW-2



### Chemistry Over Time At SW-3





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**APPENDIX J**

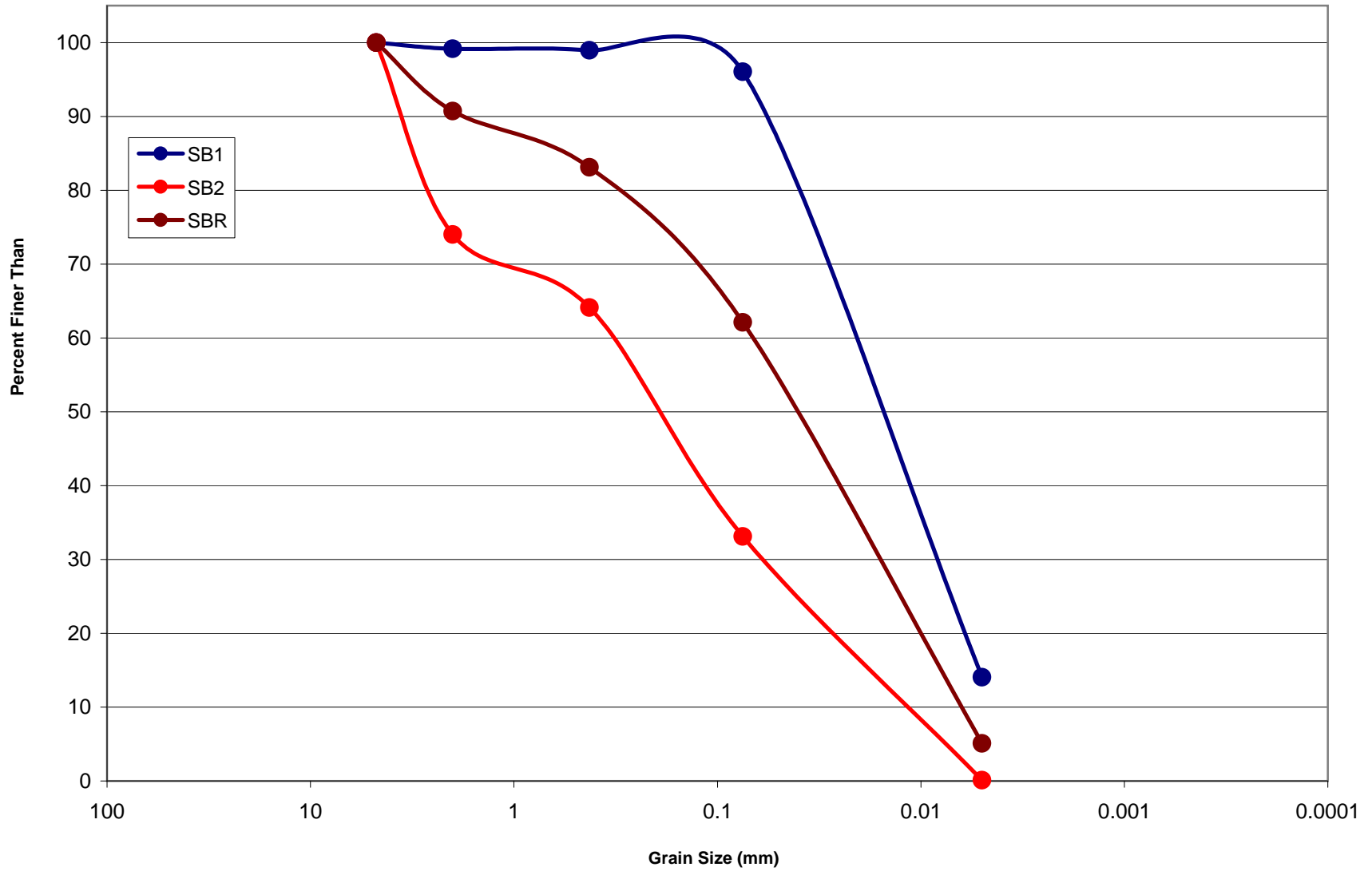
**Sediment Sampling Data**

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2012 Grain Size Distribution



**Table 7: Sediment Sampling Analysis**

Parameter	Laboratory Detection Limit	Lowest Effect Level	Severe Effect Level	Measured Concentration											
				Units	SB-1										
					9/24/2001	8/30/2003	8/23/2004	9/2/2005	8/30/2006	8/15/2007	8/20/2008	8/18/2009	8/22/2010	8/25/2011	8/31/2012
Aluminum	20			ug/g	573	1710	3930	7500	800	970	1300	640	590	530	790
Ammonia	40	100		ug/g	1	<b>196</b>	<b>280</b>	69.9	< 25	25	< 25	< 25	<25	<25	<25
Arsenic	0.2	6	33	ug/g	1.9	0.7	1.0	NA	< 1	<1	<1	< 1	<1	<1	<1
Cadmium	0.5	0.6	10	ug/g	< 0.04	< 0.5	< 0.5	< 0.3	< 0.3	<0.3	<0.3	< 0.3	<0.3	<0.3	<0.3
Chromium	1	26	110	ug/g	1.15	4	9	21	2.5	3.3	3.5	1.3	1.3	1.2	1.8
Cobalt	2	50		ug/g	< 0.04	< 2	3	5.4	0.9	0.6	1.1	< 0.5	<0.5	<0.5	NA
Copper	1	16	110	ug/g	< 0.04	2	7	<b>30</b>	2.6	2.2	2.5	1.8	0.7	0.8	1.8
Iron	50	2%	4%	%	0.07	0.26	0.47	1.10	0.15	0.16	0.2	0.11	0.8	0.065	0.09
Lead	5	31	250	ug/g	0.93	5	11	12	2	2	3	2	2	2	2
Manganese	1	460	1100	ug/g	6.75	59	62	110	17	13	21	13	10	10	12
Mercury	0.01	0.2	2	ug/g	< 0.04	<0.01	0.02	< 0.05	< 0.05	<0.05	<0.05	< 0.05	<0.05	<0.05	<0.05
Nickel	2	16	75	ug/g	1.07	3	8	<b>22</b>	1.7	1.3	2.7	1.1	0.9	0.7	1.1
Oil & Grease	100	0.15%	-	%	< 0.006	< 0.010	0.010	<b>0.400</b>	0.006	0.014	0.047	0.04	<b>0.46</b>	<0.01	0.049
Phosphorous	20	600	2000	ug/g	186	<b>956</b>	<b>882</b>	<b>920</b>	530	NA	NA	300	540	<b>1000</b>	<b>660</b>
Total Kjeldahl Nitrogen	60	550	4800	ug/g	127	<b>1450</b>	<b>2180</b>	<b>6330</b>	510	<b>724</b>	<b>553</b>	343	163	131	524
Total Organic Carbon	0.05	1%	10%	%	0.32	0.76	<b>2.39</b>	<b>7.8</b>	0.64	0.53	<b>1.3</b>	0.51	0.002	0.14	0.41
Zinc	5	120	820	ug/g	< 0.04	9	18	57	6	4	8	4	<3	<3	3

**Bold** Denotes Exceedance in Lowest Effect Level

Underline Denotes Exceedance in Severe Effect Level

NA - Not Analyzed

**Table 7: Sediment Sampling Analysis**

Parameter	Laboratory Detection Limit	Lowest Effect Level	Severe Effect Level	Units	Measured Concentration									
					SB-2									
					9/24/2001	8/30/2003	8/23/2004	9/2/2005	8/30/2006	8/15/2007	8/20/2008	8/18/2009	8/25/2011	8/31/2012
Aluminum	20			ug/g	1424	14500	9930	4100	6800	3400	3100	4200	2400	3200
Ammonia	40	100		ug/g	7.3	<b>1590</b>	<b>887</b>	1.09	< 25	<30	<25	< 25	<25	<25
Arsenic	0.2	6	33	ug/g	1.4	3.8	1.5	NA	1	<1	<1	< 1	1	<1
Cadmium	0.5	0.6	10	ug/g	< 0.04	0.5	< 0.5	< 0.3	< 0.3	<0.3	<0.3	< 0.3	<0.3	<0.3
Chromium	1	26	110	ug/g	2.53	26	18	8.4	15	8.7	8.8	11	7.9	9.9
Cobalt	2	50		ug/g	< 0.04	13	5	4.3	5.9	3.4	2.8	3.7	2	NA
Copper	1	16	110	ug/g	< 0.04	16	14	12	9	13	5.4	11	7	8.5
Iron	50	2%	4%	%	0.27	<b>2.89</b>	1.24	0.85	1.3	0.57	0.6	0.79	0.65	0.67
Lead	5	31	250	ug/g	0.93	12	11	5.1	5	3	3	4	3	3
Manganese	1	460	1100	ug/g	6.75	<b>2920</b>	235	310	210	89	61	88	72	69
Mercury	0.01	0.2	2	ug/g	< 0.04	0.1	0.09	< 0.05	< 0.05	<0.05	<0.05	< 0.05	<0.05	<0.05
Nickel	2	16	75	ug/g	2.18	<b>21</b>	14	6.9	11	6	5.3	7.8	4.5	5.5
Oil & Grease	100	0.15%	-	%	0.020	<b>0.336</b>	<b>0.273</b>	<b>0.200</b>	0.003	0.028	<0.01	0.02	<0.01	0.05
Phosphorous	20	600	2000	ug/g	228	998	583	440	570	NA	NA	300	290	330
Total Kjeldahl Nitrogen	60	550	4800	ug/g	<b>686</b>	<b>8900</b>	<b>7700</b>	<b>1180</b>	120	142	149	87	65	206
Total Organic Carbon	0.05	1%	10%	%	0.91	<b>9.94</b>	<b>17</b>	<b>4.1</b>	0.19	0.78	0.84	0.03	0.24	0.2
Zinc	5	120	820	ug/g	10.8	88	45	27	47	17	15	22	12	14

**Bold** Denotes Exceedance in Lowest Effect Level

Underline Denotes Exceedance in Severe Effect Level

NA - Not Analyzed

**Table 7: Sediment Sampling Analysis**

Parameter	Laboratory Detection Limit	Lowest Effect Level	Severe Effect Level	Measured Concentration												
				Units	SB-R											
					9/24/2001	8/30/2003	8/23/2004	9/2/2005	8/30/2006	8/15/2007	8/20/2008	8/18/2009	8/25/2010	8/25/2011	8/31/2012	
Aluminum	20			ug/g	939	1460	829	1000	710	660	760	580	860	960	840	
Ammonia	40	100		ug/g	2.2	<b>322</b>	98	1.99	< 25	<30	<25	< 25	<25	<25	<25	
Arsenic	0.2	6	33	ug/g	2.92	0.4	< 0.2	NA	< 1	<1	<1	< 1	<1	<1	<1	
Cadmium	0.5	0.6	10	ug/g	< 0.04	<0.5	< 0.5	< 0.3	< 0.3	<0.3	<0.3	< 0.3	<0.3	<0.3	<0.3	
Chromium	1	26	110	ug/g	3.02	4	3	3	2	1.6	2	1.5	2.8	2.7	2.4	
Cobalt	2	50		ug/g	0.86	<2	<2	0.9	0.6	<0.5	0.6	< 0.5	0.6	0.7	NA	
Copper	1	16	110	ug/g	< 0.04	3	1	4.8	2.1	1.3	1	6	1.2	1.3	1.5	
Iron	50	2%	4%	%	0.20	0.25	0.14	0.19	0.12	0.1	0.16	0.12	1.6	0.2	0.14	
Lead	5	31	250	ug/g	2.49	6	6	5	4	3	4	2	3	3	3	
Manganese	1	460	1100	ug/g	< 0.04	65	27	23	12	13	34	12	19	22	17	
Mercury	0.01	0.2	2	ug/g	< 0.04	< 0.01	<u>6</u>	< 0.05	< 0.05	<0.05	<0.05	< 0.05	<0.05	<0.05	<0.05	
Nickel	2	16	75	ug/g	2.3	4	< 2	3.3	1.7	1.2	1.6	1.3	2.3	2.2	1.9	
Oil & Grease	100	0.15%	-	%	0.048	0.014	< 0.010	0.052	0.0025	0.024	<0.01	0.04	<b>0.2</b>	<0.01	0.026	
Phosphorous	20	600	2000	ug/g	359	575	385	370	410	NA	NA	170	520	570	520	
Total Kjeldahl Nitrogen	60	550	4800	ug/g	454	<b>1960</b>	336	<b>666</b>	348	120	<b>732</b>	163	402	191	339	
Total Organic Carbon	0.05	1%	10%	%	<b>3.05</b>	<b>2.28</b>	0.67	<b>1.8</b>	0.61	0.48	0.71	0.33	0.005	0.46	0.45	
Zinc	5	120	820	ug/g	7.43	15	6	12	8	5	5	9	8	7	6	

**Bold** Denotes Exceedance in Lowest Effect Level

Underline Denotes Exceedance in Severe Effect Level

NA - Not Analyzed



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**APPENDIX K**

**Eng-Tech 2014 Report**

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Phone: (204) 233-1694 Fax: (204) 235-1579  
E-mail: eng\_tech@mts.net  
www.eng-tech.ca

August 8, 2014

File No. 14-415-01

City of Kenora  
60 Fourteenth Street North  
Kenora, Ontario  
P9N 4M9

**ATTENTION:** Mukesh Pokharel

**RE:** In-Situ Shear Strength Testing and Piezometer Installation – Kenora Landfill – Jones Road, Ontario

## INTRODUCTION

ENG-TECH Consulting Limited (ENG-TECH) completed in-situ shear strength testing of the soils and supervised installation of vibrating wire piezometers within the new stage area in Cell "A". The purpose of the testing was to determine the in-situ shear strengths and pore water pressures in order to verify that values obtained are as previously recommended in our report.

## Scope of Work

ENG-TECH completed the following scope of work:

- Reviewed existing information related to the project and objections.
- Drilled four (4) bore holes then completed field vane tests at predetermined depths.
- Installed two (2) vibrating wire piezometers within each borehole as per Schedule H of the Provisional Certificate of Approval No. A1612018.
- Collected soil samples at selected depths and compared these samples with existing information.
- A laboratory testing program on select soil samples.
- A report on the results and the in-situ shear strength values.

## TEST HOLE DRILLING, SOIL SAMPLING, and LABORATORY TESTING

ENG-TECH supervised the drilling of four (4) test holes (TH1 to TH4) on July 11, 2014 at the locations shown on Figure 1, attached. The test holes were drilled using a Acker SS track mounted drill rig equipped with 125 mm diameter solid stem continuous flight augers owned and operated by Maple Leaf Drilling Ltd. The test holes were advanced to various depths and upon the completion of drilling two (2) vibrating wire piezometers were installed in each test hole at various depths as shown on Figure 2.

The soil stratigraphy was visually classified using the modified Unified Soil Classification System (USCS) at the time of drilling. Soil samples were collected off the auger flights at selected depths on an as required basis. All soil samples were retained for testing in ENG-TECH's Winnipeg laboratory.

Moisture contents were determined on eight (8) soil samples collected, with two (2) Atterberg Limits and one (1) hydrometer test completed on select samples. Six (6) field shear vane tests were conducted at

various depths in the clay. Attached are test hole summary logs showing the moisture contents, Atterberg Limits, field test results, and one (1) particle size analysis report.

## STRATIGRAPHY

The stratigraphy at the site consisted of waste fill, sand fill, peat, and clay to the depths explored. The waste fill was 1.8 to 2.7 m thick and consisted of large objects which limited the ability to use hollow stem augers to the required depths. The sand fill in TH2 to TH4 was 1.2 to 3.6 m thick, black, moist, loose, fine grained and rubble like plastic bags and waste material. The peat was between 0.6 to 3.0 m thick and was brown, moist, firm to soft and contained trace wood pieces & organics. The clay was grey, moist to wet, highly plastic, soft to firm and contained trace to some silt, trace gravel, with more silt encountered at TH2.

Groundwater was encountered at depth ranging from 3.0 to 2.1 m below grade at TH1 and TH2, respectively, and 2.4 to 2.2 m at TH3 and TH4, respectively.

A summary of the subsurface conditions are shown on the test hole summary logs.

## RECOMMENDATION

The in-situ shear strength values ranged from 20 to 31 kPa are above the minimum recommended 5.5 kPa value by R. Kerry Rowe Inc. in their report *Kenora Landfill*, dated February 21, 1997. Based on the soil stratigraphy and shear strength values obtained on site, ENG-TECH concluded that the results are compliance with the values as stated in our previous report (File No. 09-415-01). Therefore, ENG-TECH supports the loading as recommended by R. Kerry Rowe Inc. for Cell A in Stage 7 of the Jones Road Landfill Development Plan.

## CLOSURE

ENG-TECH trusts the above is all the information you require. If you have any questions or require additional information, please contact the undersigned.

Sincerely,  
ENG-TECH Consulting Limited



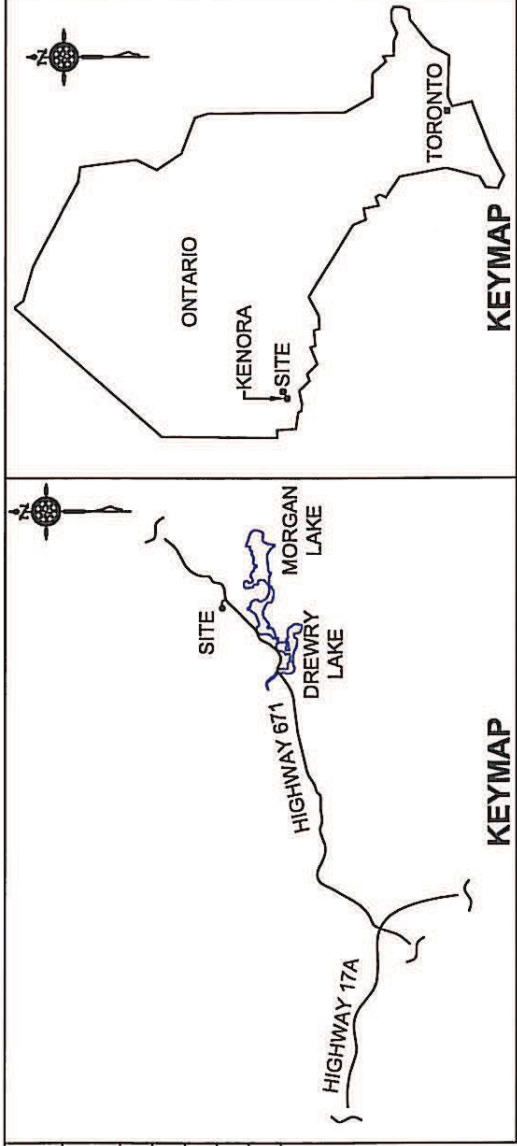
Clark Hryhoruk, M.Sc., P.Eng.  
Principal, Geotechnical Engineer

CDH/wg



Attachments:      Figure 1 – Site Plan and Test Hole Locations  
                            Figure 2 – Hydrogeological Cross Section Profile  
                            Modified Unified Classification System for Soils  
                            Test Hole Summary Logs (4)  
                            Particle Size Analysis Report (1)

TEST HOLE LOCATION TABLE	
HOLE #	GPS COORDINATES OF TEST HOLES JULY 11, 2014
	UTM 15U
TH 1	0412622 5521378
TH 2	0412664 5521335
TH 3	0412664 5521293
TH 4	0412756 5521258



**LEGEND**

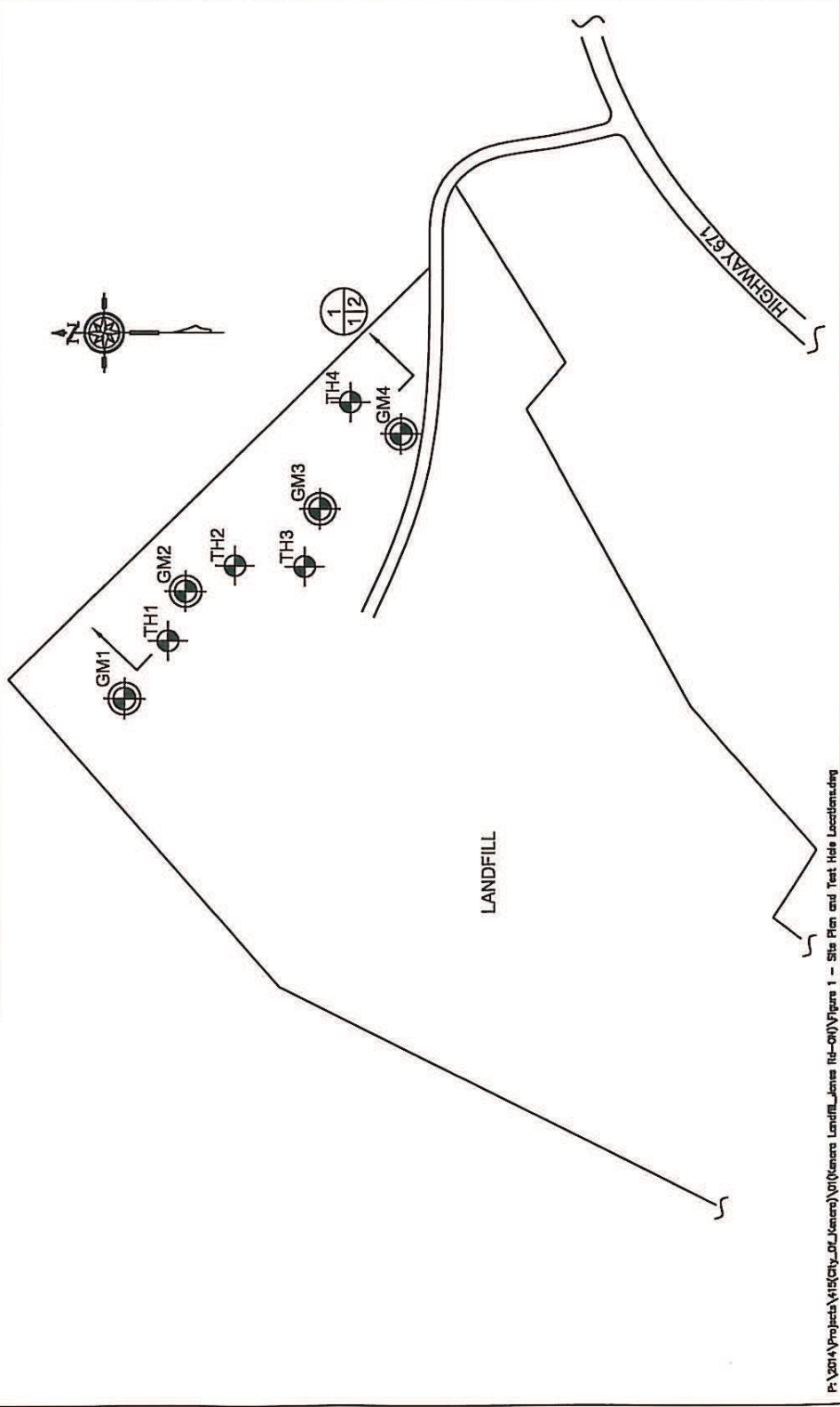
TH1 TEST HOLE AND INSTALLED VW PIEZOMETER

GM1 EXISTING PIEZOMETER

SECTION/DETAIL No.

SECTION/DETAIL PAGE LOCATION

SECTION / DETAIL PAGE REFERENCE



NO.	DATE	ISSUE / REVISION
0	Aug. 8/14	report

6 - 854 Marion Street  
Winnipeg, MB R2J 0K4  
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**ENG-TECH**  
CONSULTING LIMITED

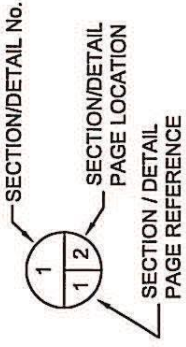
ENG. STAMP:

**APEGNI**  
Certificate of Authorization  
ENG-TECH Consulting Limited  
No.2475 Expiry: April 30, 2015

CLIENT:	CITY OF KENORA
PROJECT:	IN-SITU SHEAR STRENGTH TESTING AND PIEZOMETER INSTALLATION - KENORA LANDFILL, JONES ROAD, ONTARIO
DWG DESCRIPTION:	SITE PLAN AND TEST HOLE LOCATIONS
SCALE:	1:3000
DRAWN BY:	MRR
DATE:	AUGUST 2014
FILE No.:	14-415-01
CLIENT DWG/FIG. No.:	
ENG-TECH DWG/FIG. No.:	
1	



**LEGEND**



DEPTH OF TEST HOLES	
TEST HOLE No.	DEPTH (m)
TH 1	9.1
TH 2	12.2
TH 3	10.7
TH 4	12.2

NO.	DATE	ISSUE / REVISION
0	Aug. 8/14	report



6 - 954 Marion Street  
Winnipeg, MB R2J 0K4  
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ENG. STAMP:



CLIENT:  
CITY OF KENORA

PROJECT:  
IN-SITU SHEAR STRENGTH TESTING  
AND PIEZOMETER INSTALLATION -  
KENORA LANDFILL, JONES ROAD,  
ONTARIO

DWG DESCRIPTION:  
HYDROGEOLOGICAL  
CROSS-SECTIONS

SCALE:  
1:1250

DRAWN BY:  
MRR

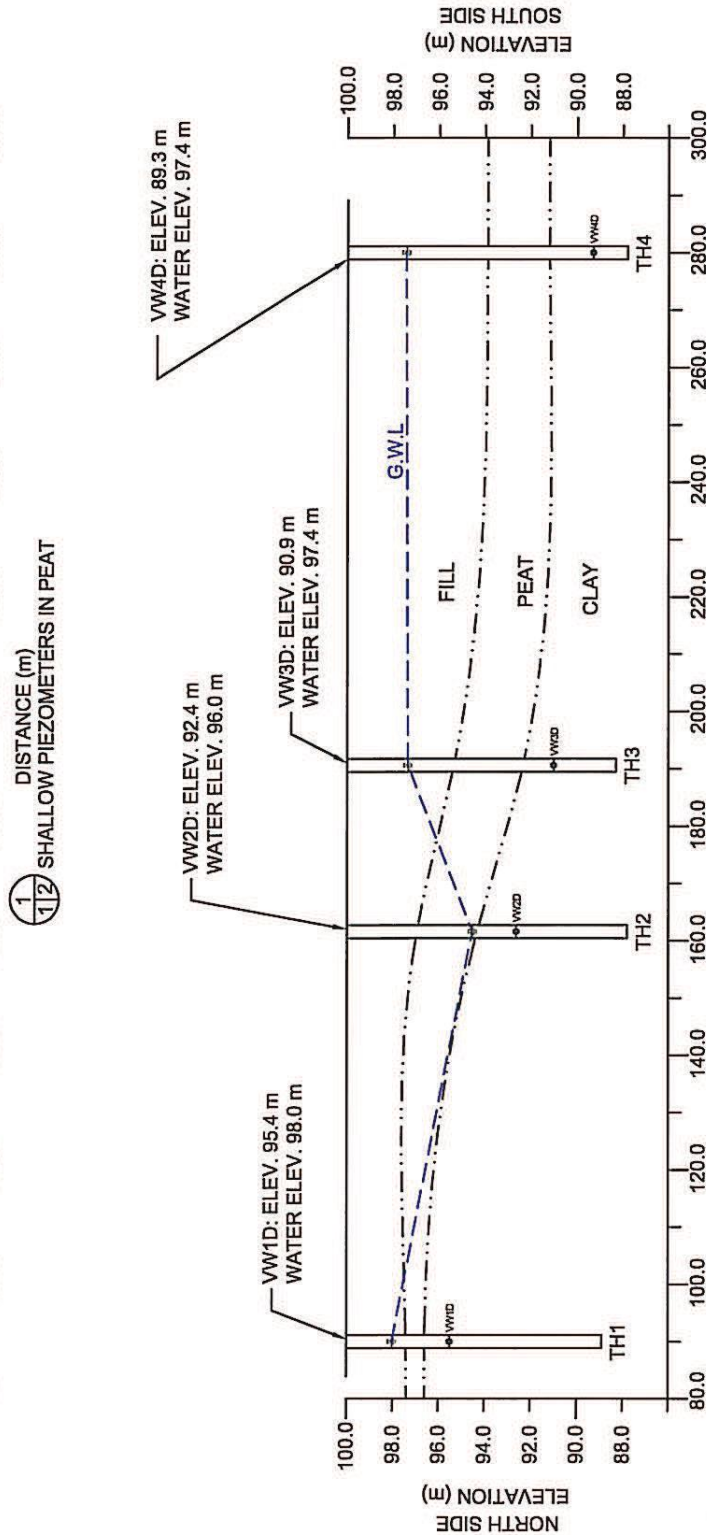
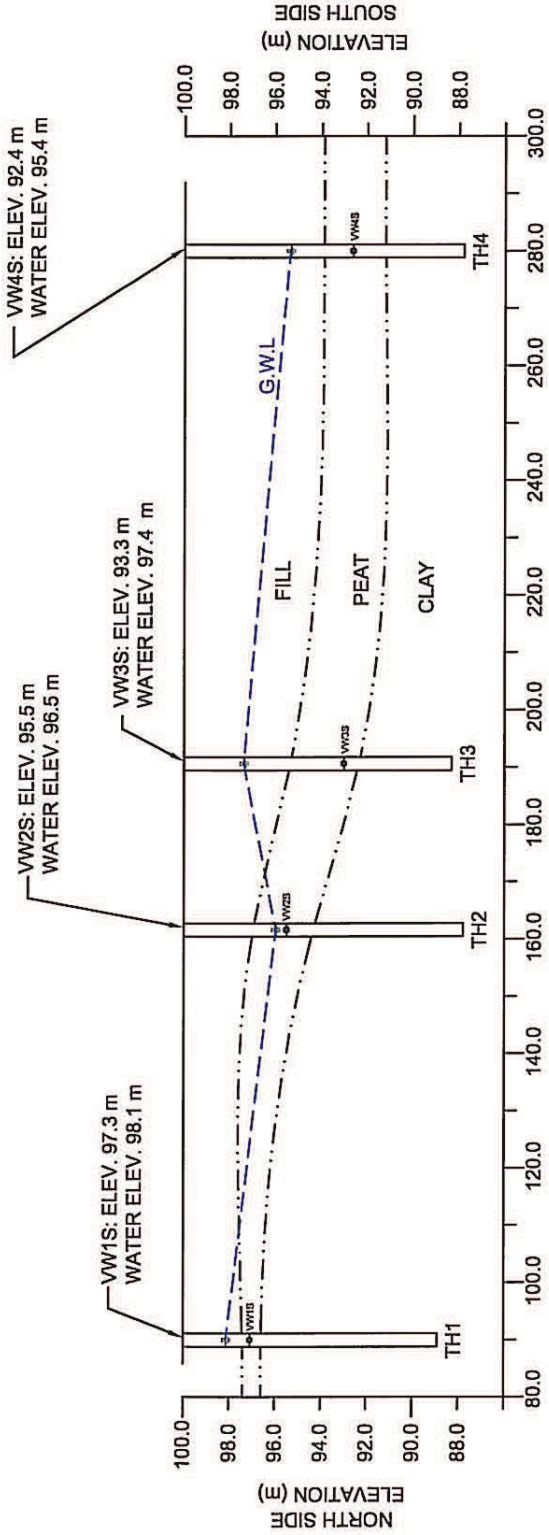
DATE:  
AUGUST 2014

FILE NO. / CLIENT DWG/FIG. NO.:

14-415-01

ENG-TECH DWG/FIG. NO.:

2



MODIFIED UNIFIED CLASSIFICATION SYSTEM FOR SOILS							
MAJOR DIVISION	GROUP SYMBOL	GRAPH SYMBOL	TYPICAL DESCRIPTION	LABORATORY CLASSIFICATION CRITERIA			
COARSE GRAINED SOILS (MORE THAN HALF BY WEIGHT LARGER THAN 75 µm)	GRAVELS MORE THAN HALF THE COARSE FRACTION LARGER THAN 4.75 mm	CLEAN GRAVELS (TRACE OR NO FINES)	GW	WELL GRADED GRAVELS, GRAVEL-SAND MIXTURES, LITTLE OR NO FINES	$C_u = \frac{D_{60}}{D_{10}} > 4$ ; $C_c = \frac{(D_{30})^2}{D_{10} \times D_{60}} = 1 \text{ TO } 3$		
		DIRTY GRAVELS (WITH SOME OR MORE FINES)	GP	POORLY GRADED GRAVELS, GRAVEL- SAND MIXTURES, LITTLE OR NO FINES	NOT MEETING ABOVE REQUIREMENTS		
			GM	SILTY GRAVELS, GRAVEL-SAND-SILT MIXTURES	ATTERBERG LIMITS BELOW "A" LINE OR P.I. LESS THAN 4		
		GC	CLAYEY GRAVELS, GRAVEL-SAND-CLAY MIXTURES	ATTERBERG LIMITS ABOVE "A" LINE AND P.I. MORE THAN 7			
	SANDS MORE THAN HALF THE COARSE FRACTION SMALLER THAN 4.75 mm	CLEAN SANDS (TRACE OR NO FINES)	SW	WELL GRADED SANDS, GRAVELLY SANDS, LITTLE OR NO FINES	$C_u = \frac{D_{60}}{D_{10}} > 6$ ; $C_c = \frac{(D_{30})^2}{D_{10} \times D_{60}} = 1 \text{ TO } 3$		
		DIRTY SANDS (WITH SOME OR MORE FINES)	SP	POORLY GRADED SANDS, GRAVELLY SANDS, LITTLE OR NO FINES	NOT MEETING ABOVE REQUIREMENTS		
			SM	SILTY SANDS, SAND-SILT MIXTURES	ATTERBERG LIMITS BELOW "A" LINE OR P.I. LESS THAN 4		
		SC	CLAYEY SANDS, SAND-CLAY MIXTURES	ATTERBERG LIMITS ABOVE "A" LINE AND P.I. MORE THAN 7			
		FINE GRAINED SOILS (MORE THAN HALF BY WEIGHT SMALLER THAN 75 µm)	SILTS BELOW "A" LINE NEGLECTIBLE ORGANIC CONTENT	LL ≤ 50%	ML	INORGANIC SILTS AND VERY FINE SANDS, ROCK FLOUR, SILTY SANDS OF SLIGHTY PLASTICITY	CLASSIFICATION IS BASED UPON PLASTICITY CHART (SEE BELOW)
				LL > 50%	MH	INORGANIC SILTS, MICACEOUS OR DIATOMACEOUS, FINE SANDY OR SILTY SOILS	
CLAYS ABOVE "A" LINE NEGLECTIBLE ORGANIC CONTENT	LL ≤ 30%		CL	INORGANIC CLAYS OF LOW PLASTICITY, GRAVELLY, SANDY OR SILTY CLAYS, LEAN CLAYS			
	30% < LL ≤ 50%		CI	INORGANIC CLAYS OF MEDIUM PLASTICITY, SILTY CLAYS			
	LL > 50%		CH	INORGANIC CLAYS OF HIGH PLASTICITY, FAT CLAYS			
ORGANIC SILTS & CLAYS BELOW "A" LINE	LL < 50%		OL	ORGANIC SILTS AND ORGANIC SILTY CLAYS OF LOW PLASTICITY			
	LL > 50%		OH	ORGANIC CLAYS OF HIGH PLASTICITY			
HIGHLY ORGANIC SOILS	PI		PEAT AND OTHER HIGHLY ORGANIC SOILS	STRONG COLOUR OR ODOUR, AND OFTEN FIBROUS TEXTURE			

ADDITIONAL SYMBOLS		PLASTIC SOILS	
TILL		SANDSTONE	
FILL		GRANITE	
TOPSOIL			
CONCRETE			
SHALE			
LIMESTONE			

MOISTURE	PLASTICITY	INTRUSIONS	CONSISTENCY	POCKET PEN (TSF)	(N)
DRY	LOW	ROOTLETS	VERY SOFT	< 2	< 2
DAMP	MEDIUM	OXIDES	SOFT	0 - 0.5	2 - 4
MOIST	HIGH	MICA	FIRM	0.5 - 1.0	4 - 8
WET		GYPSUM	STIFF	1.0 - 2.0	8 - 15
		ETC.	VERY STIFF	2.0 - 4.0	15 - 30
			HARD	> 4.0	> 30

TSF x 95.8 = kPa (q<sub>u</sub>)      S<sub>u</sub> = 1/2 x q<sub>u</sub>

SOIL DESCRIPTIONS		
TRACE: 0 - 10%	BOULDERS: > 200 mm	COARSE SAND: 2 - 4.75 mm
SOME: 10 - 20%	COBBLES: 75 - 200 mm	MEDIUM SAND: 0.425 - 2 mm
WITH: 20 - 35%	COURSE GRAVEL: 19 - 75 mm	FINE SAND: 0.075 - 0.425 mm
AND: 35 - 50%	FINE GRAVEL: 4.75 - 75 mm	FINES: < 0.075 mm

GRANULAR SOILS				
MOISTURE	DENSITY	GRADATION	INTRUSIONS	SPT (N)
DRY	VERY LOOSE	POORLY	ROOTLETS	0 - 4
DAMP	LOOSE	WELL	OXIDES	4 - 10
MOIST	MED. DENSE		MICA	10 - 30
WET	DENSE		FINES	30 - 50
	VERY DENSE		ETC.	> 50

DEFINITIONS	
LL = LIQUID LIMIT	C <sub>c</sub> = COMPRESSION INDEX
P.I. = PLASTICITY INDEX	PL = PLASTIC LIMIT
C <sub>u</sub> = COEFFICIENT OF UNIFORMITY	
q <sub>u</sub> = UNCONFINED COMPRESSIVE STRENGTH	
S <sub>u</sub> = UNDRAINED SHEAR STRENGTH	

6 - 854 Marion Street  
Winnipeg, MB R2J 0K4  
Phone: (204) 233-1694  
Fax: (204) 235-1578

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Engineering And Testing  
Solutions That Work For You

**Test Hole #: TH1**  
**Client:** City of Kenora  
**Site:** Kenora, Ontario  
**Location:** See Figure 1  
**Project:** Insitu Shear Strength Test & Piezometer Installation - Jones Road, ON

**File No.:** 14-415-01  
**Date Drilled:** July 11, 2014  
**Grade Elevation:** 100.0 m  
**Water Elevation:** 97.0 m

SUBSURFACE PROFILE			SAMPLE DATA		UNDRAINED SHEAR STRENGTH (kPa)		GRAIN SIZE DISTRIBUTION %											
Depth (m)	Soil Symbol	Description	Elevation (m)	Sample No.	Sample Type	Shear Value	Rebound	Water Content (%)										
								PL	X	LL		GRAVEL	SAND	SILT	CLAY			
0		Ground Surface	100.0															
0		<b>Waste Fill</b>																
3		<b>Peat</b> - dark brown, moist, firm to soft, trace processed wood pieces, and organics.	97.0															
4		<b>Clay (CH)</b> - grey, moist, firm, highly plastic, trace to some silt, trace gravel.	96.0															
5			95.0	S1														
6			94.0	S2														
9.1		<b>End of Test Hole</b> - end of test hole at 9.1 m below grade. - water seepage and sloughing observed at 3.0 m below grade. - piezometer installed at 2.7 and 4.6 m below grade.	91.0															
10			90.0															
11			89.0															

ENG-TECH Consulting Limited

Logged by: WG

Reviewed by:

Drilled By: Maple Leaf Drilling

Drill Rig: Acker SS

Auger Size: 125 mm

Completion Depth: 9.1 m

Completion Elevation: 90.9 m

Sheet: 1 of 1

SAMPLE TYPE



SPLIT BARREL



SHELBY TUBE



AUGER CUTTINGS



SPLIT SPOON



Engineering And Testing  
Solutions That Work For You

**Test Hole #: TH2**  
**Client:** City of Kenora  
**Site:** Kenora, Ontario  
**Location:** See Figure 1  
**Project:** Insitu Shear Strength Test & Piezometer Installation - Jones Road, ON

**File No.:** 14-415-01  
**Date Drilled:** July 11, 2014  
**Grade Elevation:** 100.0 m  
**Water Elevation:** 97.9 m

SUBSURFACE PROFILE			SAMPLE DATA		UNDRAINED SHEAR STRENGTH (kPa)		GRAIN SIZE DISTRIBUTION %						
Depth (m)	Soil Symbol	Description	Elevation (m)	Sample No.	Sample Type	Shear Value	Rebound	Water Content (%)					
								PL	X	LL			
								GRAVEL	SAND	SILT	CLAY		
0		Ground Surface	100.0										
0		<b>Waste Fill</b>											
1			99.0										
2		<b>Sand Fill</b>	98.0										
2		- black, moist, medium dense, loose, fine grained, trace waste material, and processed wood pieces.											
3		<b>Peat</b>	97.0										
3		- brown, moist, soft, trace processed wood pieces & organics.											
4			96.0										
5			95.0										
6		<b>Clay (CH)</b>	94.0	S1		25	21						
6		- grey, moist to wet, soft, highly plastic, and silt.											
7			93.0	S2		22	20						
7										0.1	3.2	46.3	50.4
8			92.0										
8		- below 8.5 m, trace to some gravel.											
9			91.0										
10			90.0										
11			89.0										
12			88.0										
12		<b>End of Test Hole</b>											
12		- end of test hole at 12.2 m below grade.											
13		- water seepage observed at 2.1 m below grade and sloughing observed at 8.5 m below grade.	87.0										
14		- piezometer installed at 4.5 and 7.5 m below grade.	86.0										

ENG-TECH Consulting Limited

Logged by: WG

Reviewed by:

Drilled By: Maple Leaf Drilling

Drill Rig: Acker SS

Auger Size: 125 mm

Completion Depth: 12.2 m

Completion Elevation: 87.8 m

Sheet: 1 of 1

SAMPLE TYPE



SPLIT BARREL



SHELBY TUBE



AUGER CUTTINGS



SPLIT SPOON



Engineering And Testing  
Solutions That Work For You

Test Hole #: TH3

Client: City of Kenora .

Site: Kenora, Ontario

Location: See Figure 1

Project: Insitu Shear Strength Test & Piezometer Installation - Jones Road, ON

File No.: 14-415-01

Date Drilled: July 11, 2014

Grade Elevation: 100.0 m

Water Elevation: 97.6 m

SUBSURFACE PROFILE			SAMPLE DATA		UNDRAINED SHEAR STRENGTH (kPa)		GRAIN SIZE DISTRIBUTION %						
Depth (m)	Soil Symbol	Description	Elevation (m)	Sample No.	Sample Type	Shear Value	Rebound	Water Content (%)					
								PL	X	LL			
								GRAVEL	SAND	SILT	CLAY		
0		Ground Surface	100.0										
0		<b>Waste Fill</b>											
1			99.0										
2			98.0										
3		<b>Sand Fill</b> - black, moist, loose, fine grained, trace plastic bags & processed wood pieces.	97.0										
4			96.0										
5		<b>Peat</b> - brown, wet, soft, trace processed wood pieces & organics.	95.0										
6		- below 6.1 m, soft to firm.	94.0										
7			93.0										
8		<b>Clay (CH)</b> - grey, moist, firm to soft, highly plastic, trace to some silt.	92.0	S1		20	14						
9			91.0	S2		31	29						
10			90.0										
11		<b>End of Test Hole</b> - end of test hole at 10.7 m below grade. - water seepage observed at 2.4 m below grade and sloughing observed at 5.2 m below grade. - piezometer installed at 6.7 and 9.1 m below grade.	89.0										
12			88.0										

ENG-TECH Consulting Limited

Logged by: WG

Reviewed by:

Drilled By: Maple Leaf Drilling

Drill Rig: Acker SS

Auger Size: 125 mm

Completion Depth: 10.7 m

Completion Elevation: 89.3 m

Sheet: 1 of 1

SAMPLE TYPE



SPLIT BARREL



SHELBY TUBE



AUGER CUTTINGS



SPLIT SPOON



Engineering And Testing  
Solutions That Work For You

**Test Hole #: TH4**

**Client:** City of Kenora

**Site:** Kenora, Ontario

**Location:** See Figure 1

**Project:** Insitu Shear Strength Test & Piezometer Installation - Jones Road, ON

**File No.:** 14-415-01

**Date Drilled:** July 11, 2014

**Grade Elevation:** 100.0 m

**Water Elevation:** 97.8 m

SUBSURFACE PROFILE			SAMPLE DATA		UNDRAINED SHEAR STRENGTH (kPa)		GRAIN SIZE DISTRIBUTION %						
Depth (m)	Soil Symbol	Description	Elevation (m)	Sample No.	Sample Type	Shear Value	Rebound	Water Content (%)					
								PL	X	LL			
								GRAVEL	SAND	SILT	CLAY		
0		Ground Surface	100.0										
0		<b>Waste Fill</b>											
1			99.0										
2			98.0										
3		<b>Sand Fill</b> - black, moist, loose, fine grained, trace plastic bags & processed wood pieces.	97.0										
4			96.0										
5			95.0										
6			94.0										
7		<b>Peat</b> - brown, moist, soft, trace processed wood pieces, and organics.	93.0										
8			92.0										
9			91.0	S1		22	18						
10		<b>Clay (CH)</b> - grey, moist, soft, highly plastic, trace to some silt, trace gravel.	90.0										
11			89.0	S2		49	37						
12			88.0										
13		<b>End of Test Hole</b> - end of test hole at 12.2 m below grade. - water seepage observed at 2.2 m below grade and sloughing observed at 9.1 m below grade. - piezometer installed at 7.6 and 10.7 m below grade.	87.0										
14			86.0										

ENG-TECH Consulting Limited

Logged by: WG

Reviewed by:

Drilled By: Maple Leaf Drilling

Drill Rig: Acker SS

Auger Size: 125 mm

Completion Depth: 12.2 m

Completion Elevation: 87.8 m

Sheet: 1 of 1

SAMPLE TYPE



SPLIT BARREL



SHELBY TUBE



AUGER CUTTINGS



SPLIT SPOON



Unit 6 - 854 Marion Street  
 Winnipeg, Manitoba  
 R2J 0K4  
 eng\_tech@mts.net  
 www.eng-tech.ca

**PARTICLE SIZE  
 ANALYSIS REPORT**

City of Kenora  
 60 Fourteenth Street N.  
 Kenora, ON  
 P9N 4M9

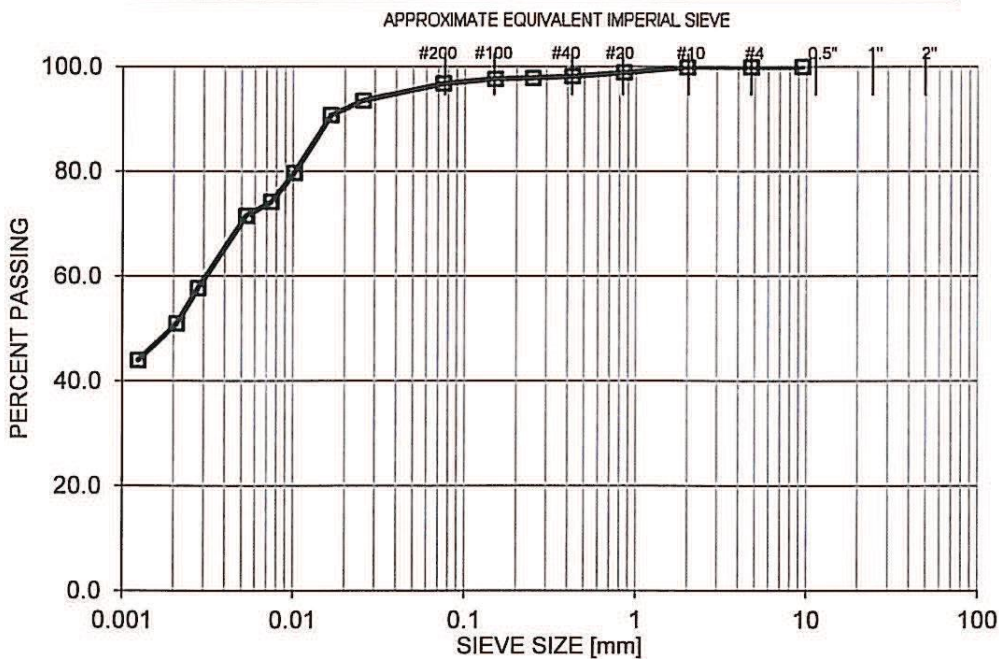
File No.: 14-415-01  
 Ref. No.: 14-415-1-1

**ATTENTION:** Mukesh Pokharel

**PROJECT:** IN-SITU SHEAR STRENGTH TEST AND PIEZOMETER INSTALLATION - KENORA LANDFILL

Test Hole No. TH2      Sample No. S2      Depth: 7.0 m  
 Sampled By: ENG-TECH      Type of Sample: Grab      Source: Project Site  
 Date Sampled: Jul 11/14      Date Received: Jul 14/14      Date Tested: Jul 16/14

CLAY	SILT	SAND			GRAVEL	
		FINE	MEDIUM	COARSE	FINE	COARSE

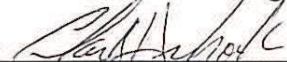


SIEVE SIZE (mm)	PERCENT PASSING
9.500	100.0
4.750	99.9
2.000	99.9
0.850	98.9
0.425	98.2
0.250	97.8
0.150	97.6
0.075	96.8
0.026	93.5
0.017	90.7
0.0102	79.7
0.0074	74.2
0.0053	71.6
0.0028	57.7
0.0021	51.0
0.0012	44.0

Percent of: GRAVEL (0.1 %), SAND (3.2 %), SILT (46.3 %), CLAY (50.4 %)  
 Sample Description:

COMMENTS:

**ENG-TECH Consulting Limited**

per   
 Clark Hryhoruk, M.Sc., P.Eng., President  
 Ph: (204) 233-1694 Fax: (204) 235-1579



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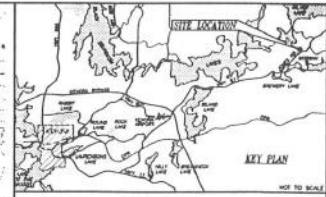
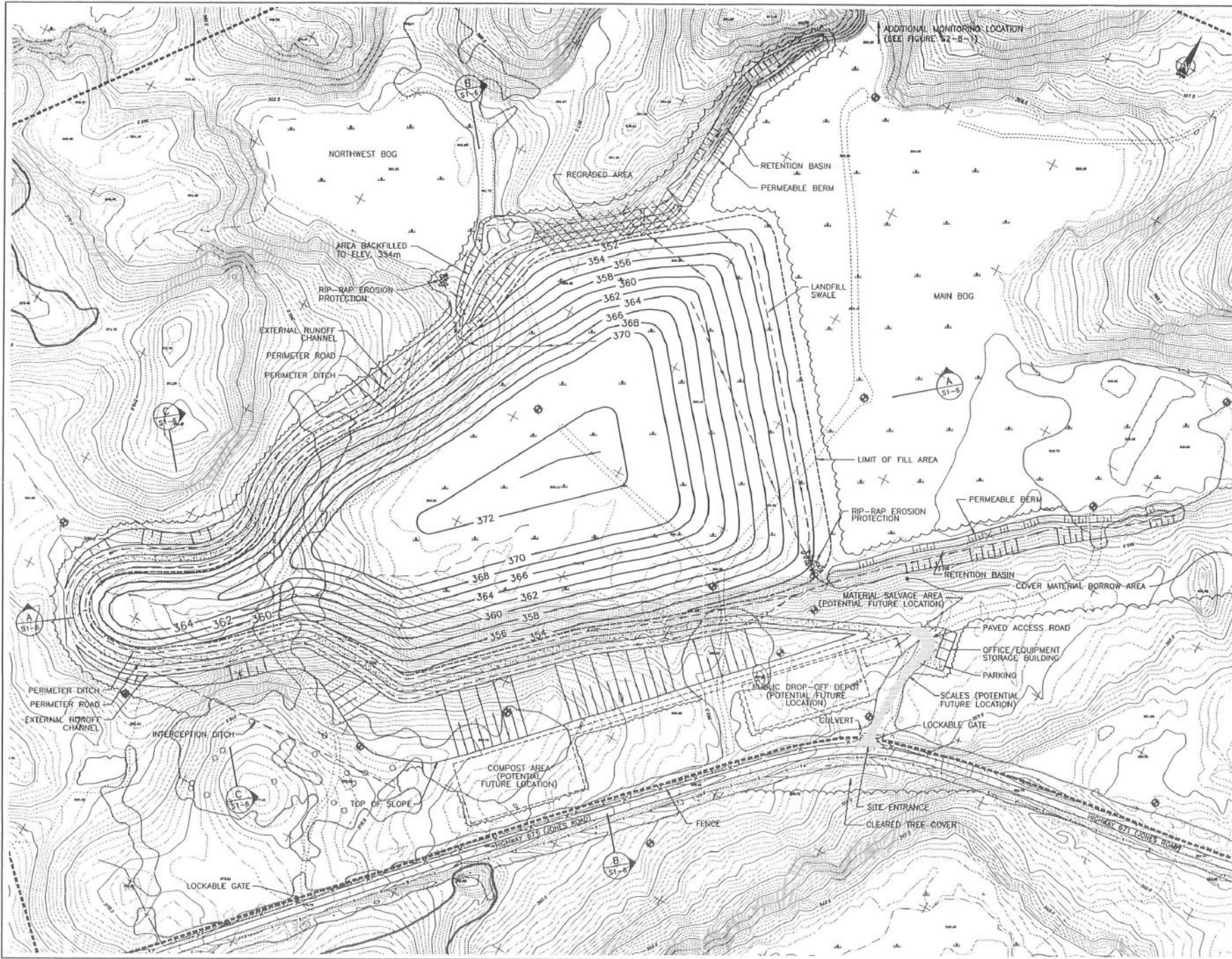
**APPENDIX L**

**Final Contours Design Drawing**

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**LEGEND**

- EXISTING CONTOURS
- STREAM
- INTERMEDIATE STREAM
- EXISTING TRAIL
- EXISTING TREE LINE
- ROCK OUTCROP
- EXISTING DITCH
- BOG
- LIMIT OF CUTBACK TO TREE LINE
- FENCE
- SITE BOUNDARY
- LIMIT OF WASTE DISPOSAL
- FINAL CONTOUR
- PAVED ACCESS ROAD
- GRANULAR PERIMETER/ACCESS ROAD
- CULVERT
- DITCH/CHANNEL
- PROPOSED FACILITIES
- POTENTIAL FUTURE FACILITIES
- GROUNDWATER MONITORING LOCATION
- GAS MONITORING PROBE

REV.	DATE	REVISIONS/ISSUES/APPROVALS	APP'D.

**Fenco MacLaren**  
Member of the BGC-LAYZELL Group

CLIENT  
**TOWN OF KENORA**

PROJECT  
**KENORA AREA WASTE MANAGEMENT MASTER PLAN  
PROPOSED KENORA AREA LANDFILL**

TITLE  
**FINAL CONTOURS**

DESIGNED BY:	T. TAYLOR	DATE:	FEBRUARY 1997
DRAWN BY:	D. MCKENZIE	SCALE:	1 : 1500
CHECKED BY:	T. TAYLOR	FILE NAME:	KEN-FCO0.DWG
PROJECT NUMBER:	015926	DRAWING NUMBER:	S1-2



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**APPENDIX M**

**Borehole Logs**

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Project Kenora WMMP No.: 015926  
 Date Drilled 96/2/12 - 96/2/14 Driller: Paddock Drilling  
 Borehole Location: See Figure S2-3-1  
 Drilling Supervised by: S. Bricks  
 Drilling Method Hollow Stem Augers to 5.9 m;  
HQ Coring to 16.9 m  
 Piezometer Details  
 Type of Pipe 51 mm Dia. Sch. 40 Threaded PVC  
 Type of Screen 51 mm Dia. Sch. 40 Threaded PVC, #10 Slot

Borehole No. BH-1/17  
 Piezometers MW-1/17

Sheet 1 of 2

- SS Split Spoon Sample    RX Rock Core
- WA Wash Sample            GR Grab Sample
- AU Auger Sample            Piezometer Screen
- CN Continuous Sample      Water Level Elev. m (with date)
- SH Shelby Tube Sample

Scale (ft.) (m)	Elev. (m GSD) Depth (m)	Geological Log		Strat. Plot	Monitor Details	Sample No.	PILOT	Sample Type	Vapour Conc.
		Description							
		Ground Surface Elevation: 350.57 m							
1 5 10 15 20 25 30 35 40 45 50 16		GRAVELLY SILTY SAND - grey-rust mottled, becoming grey below 2.3 m - with cobbles and some boulders - locally TILL-LIKE - wet below 0.8 m - generally compact; locally dilatant, becoming more dense with depth  - auger refusal at 5.9 m; changed to HQ coring   - becomes very sandy, less till-like below 12.2 m			Holeplug and Cave				
						1		SS	14
						2		SS	7
						3		SS 41/23cm	
						4		SS 55/15cm	
						5		SS	67
						6		SS	102
						7		SS 100/25cm	
						1		RX	
						2		RX	
						3		RX	
						4		RX	
						5		RX	
						6		RX	
						7		RX	
Borehole Record	<u>S. Bricks</u> Prepared by		<u>H. Jackson</u> Checked by		<u>Fenco MacLaren</u>			97/2/19	

Project **Kenora WMMP** No.: **015926**

Borehole No. **BH-1/17**

Continuation Sheet







Sheet 2 of 2

Scale (ft.) (m)	Elev. (m GSD) Depth (m)	Geological Log		Strat. Plot	Monitor Details	Sample No.	Sample Type	Vapour Conc.
		Description						
55 17	333.7 16.9	End of Borehole @ 16.9 m		[Stratigraphic Column Symbol]				
60 18								
65 19								
70 20								
75 21								
80 22								
85 23								
90 24								
95 25								
100 26								
105 27								
110 28								

Borehole Record

Fenco MacLaren

97/2/19



Project <u>Kenora WMMP</u> No.: <u>015926</u>		Borehole No. <u>BH-2/10</u>						
Date Drilled <u>96/1/30 - 96/1/30</u> Driller: <u>Paddock Drilling</u>		Piezometers <u>MW-2/9</u>						
Borehole Location: <u>See Figure S2-3-1</u>		Sheet 1 of 1						
Drilling Supervised by: <u>S. Bricks</u>		SS Split Spoon Sample RX Rock Core						
Drilling Method <u>Hollow Stem Augers to 9.6 m</u>		WA Wash Sample GR Grab Sample						
Piezometer Details		AU Auger Sample  Piezometer Screen						
Type of Pipe <u>51 mm Dia. Sch. 40 Threaded PVC</u>		CN Continuous Sample  Water Level Elev. m (with date)						
Type of Screen <u>51 mm Dia. Sch. 40 Threaded PVC, #10 Slot</u>		SH Shelby Tube Sample						
Scale (ft.) (m)	Elev. (m GSD) Depth (m)	Geological Log Description	Strat. Plot	Monitor Details	Sample No.	Plot	Sample Type	Vapour Conc.
		Ground Surface Elevation: 350.33 m						
1		PEAT/ORGANICS - frozen to 0.6 m, wet below - dark brown to black			1		GRAB	
5					2		SS	1
10	347.2	CLAYEY SILT - bluish grey - very soft - increasing silt content with depth, becoming SILT, SOME CLAY below 5.5 m - wet		Holeplug	3		SS	push
15	344.0				4		SS	push
20	6.3	SAND - trace to some silt - grey - generally fine to medium, locally medium to coarse - wet			5		SS	push
25					6		SS	2
30	341.7	SILT-SAND TILL - grey - gravelly - compact; wet		Cave	7		SS	3
35	8.6				8		SS	6
40	340.7	End of Borehole @ 9.6 m			9		SS	2
45	9.6				10		SS	24
50								
16								
Borehole Record	<u>S. Bricks</u> Prepared by		<u>H. Jackson</u> Checked by		<u>Fenco MacLaren</u> 97/2/19			

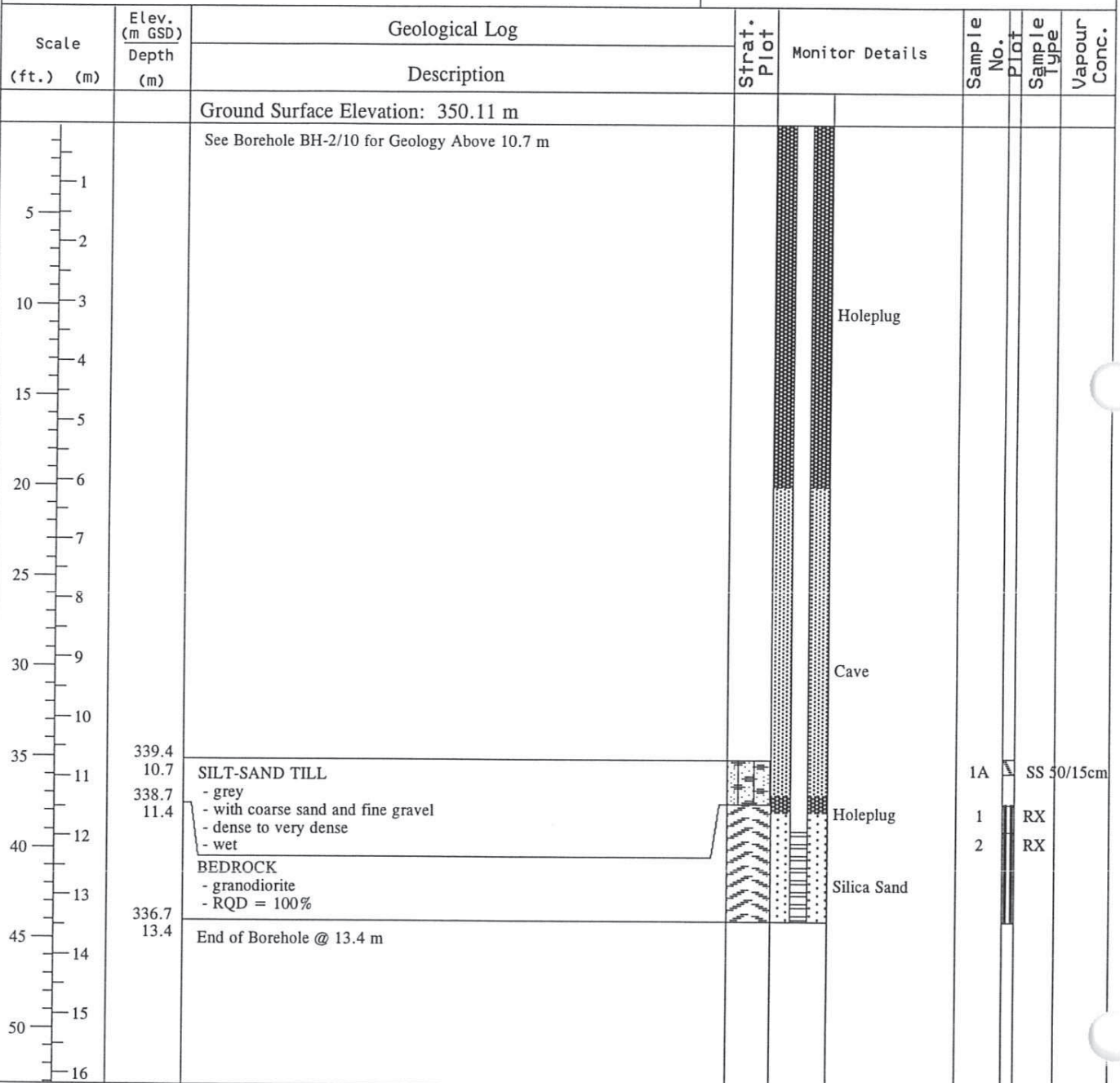
Project Kenora WMMP No.: 015926  
 Date Drilled 96/2/7 - 96/2/7 Driller: Paddock Drilling  
 Borehole Location: See Figure S2-3-1  
 Drilling Supervised by: S. Bricks  
 Drilling Method Hollow Stem Augers to 11.4 m;  
HQ Coring to 13.3 m

Borehole No. BH-2/13  
 Piezometers MW-2/13

Sheet 1 of 1

Piezometer Details  
 Type of Pipe 51 mm Dia. Sch. 40 Threaded PVC  
 Type of Screen 51 mm Dia. Sch. 40 Threaded PVC, #10 Slot

SS Split Spoon Sample RX Rock Core  
 WA Wash Sample GR Grab Sample  
 AU Auger Sample  Piezometer Screen  
 CN Continuous Sample  Water Level Elev. m (with date)  
 SH Shelby Tube Sample



Borehole Record S. Bricks Prepared by H. Jackson Checked by Fenco MacLaren  
 97/2/19

Project Kenora WMMP No.: 015926  
 Date Drilled 96/1/27 - 96/1/27 Driller: Paddock Drilling  
 Borehole Location: See Figure S2-3-1  
 Drilling Supervised by: S. Bricks  
 Drilling Method Solid Stem Augers to 10.7 m

Borehole No. BH-3/11  
 Piezometers MW-3/8

Sheet 1 of 1

Piezometer Details  
 Type of Pipe 51 mm Dia. Sch. 40 Threaded PVC  
 Type of Screen 51 mm Dia. Threaded PVC, #10 Slot with Geosock

- SS Split Spoon Sample    RX Rock Core
- WA Wash Sample         GR Grab Sample
- AU Auger Sample         Piezometer Screen
- CN Continuous Sample    Water Level Elev. m (with date)
- SH Shelby Tube Sample

Scale (ft.) (m)	Elev. (m GSD) Depth (m)	Geological Log		Strat. Plot	Monitor Details	Sample No.	Plot	Sample Type	Vapour Conc.
		Description							
		Ground Surface Elevation: 350.63 m							
1	350.2 0.5	PEAT/ORGANICS - black							
5		SAND TILL - some silt to silty, some gravel to gravelly - brown, becoming grey below 1.5 m - evidence of roots to 2.7 m - wet - generally compact			Holeplug	1	/	SS	11
2						2	/	SS	15
3						3	/	SS	14
4						4	/	SS	19
5					Cave	5	/	SS	9
6						6	/	SS	18
7						7	/	SS	30
8					Geosock on Screen	8	/	SS	20
9						9	/	SS	47
10		- augering continued to 10.7 m, however, due to bouldery conditions and flowing sands, samples were not recovered below 8 m.			Cave				
11	340.0 10.7	End of Borehole @ 10.7 m							



Borehole Record	<u>S. Bricks</u> Prepared by	<u>H. Jackson</u> Checked by	<u>Fenco MacLaren</u> 97/2/19
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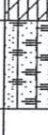
Project Kenora WMMP No.: 015926  
 Date Drilled 96/1/26 - 96/1/26 Driller: Paddock Drilling  
 Borehole Location: See Figure S2-3-1  
 Drilling Supervised by: S. Bricks  
 Drilling Method Hollow Stem Augers to 5.8 m

Borehole No. BH-4/6  
 Piezometers MW-4/6

Sheet 1 of 1

Piezometer Details  
 Type of Pipe 51 mm Dia. Sch. 40 Threaded PVC  
 Type of Screen 51 mm Dia. Sch. 40 Threaded PVC, #10 Slot

SS Split Spoon Sample RX Rock Core  
 WA Wash Sample GR Grab Sample  
 AU Auger Sample  Piezometer Screen  
 CN Continuous Sample  Water Level Elev. m (with date)  
 SH Shelby Tube Sample

Scale (ft.) (m)	Elev. (m GSD) Depth (m)	Geological Log		Strat. Plot	Monitor Details	Sample No.	PILOT	Sample Type	Vapour Conc.
		Description							
		Ground Surface Elevation: 350.75 m							
1		PEAT/ORGANICS - black, wet				1		SS	push
5	348.5	CLAYEY SILT - green-brown to grey-brown - with trace, very thin silty fine sand layers - soft to very soft - wet			Holeplug	2		SS	push
10	346.2	SAND TILL - some silt, some gravel and cobbles - grey - loose - wet			Silica Sand and Cave	3		SS	5
15	4.6					4		SS	5
20	345.0	End of Borehole @ 5.8 m				5		SS	push
25	5.8					6		SS	11
30						7		SS	11
35									
40									
45									
50									
16									
Borehole Record		<u>S. Bricks</u> Prepared by		<u>H. Jackson</u> Checked by		<u>Fenco MacLaren</u> 97/2/19			



Project <u>Kenora WMMP</u> No.: <u>015926</u>	Borehole No. <u>BH-4/18</u>
Date Drilled <u>96/2/9 - 96/2/9</u> Driller: <u>Paddock Drilling</u>	Piezometers <u>MW-4/18</u>
Borehole Location: <u>See Figure S2-3-1</u>	Sheet 1 of 2
Drilling Supervised by: <u>S. Bricks</u>	
Drilling Method <u>Hollow Stem Augers to 5.8 m;</u>	
<u>HQ Coring to 18.3 m</u>	
Piezometer Details	
Type of Pipe <u>51 mm Dia. Sch. 40 Threaded PVC</u>	
Type of Screen <u>51 mm Dia. Sch. 40 Threaded PVC, #10 Slot</u>	

- |                       |                                 |
|-----------------------|---------------------------------|
| SS Split Spoon Sample | RX Rock Core                    |
| WA Wash Sample        | GR Grab Sample                  |
| AU Auger Sample       | Piezometer Screen               |
| CN Continuous Sample  | Water Level Elev. m (with date) |
| SH Shelby Tube Sample |                                 |

Scale (ft.) (m)	Elev. (m GSD) Depth (m)	Geological Log Description	Strat. Pilot	Monitor Details	Sample No.	Pilot	Sample Type	Vapour Conc.
		Ground Surface Elevation: 350.56 m						
		See Borehole Log BH-4/6 for Descriptions to 5.8 m		Holeplug				
	344.8 5.8	SAND to SILTY SAND TILL - with cobbles, some boulders - grey to greenish grey		Cave	1 2 3 4 5 6		RX RX RX RX RX RX	

Borehole Record	<u>S. Bricks</u> Prepared by	<u>H. Jackson</u> Checked by	<u>Fenco MacLaren</u> 97/2/19
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Project **Kenora WMMP** No.: **015926**

Borehole No. **BH-4/18**

Continuation Sheet



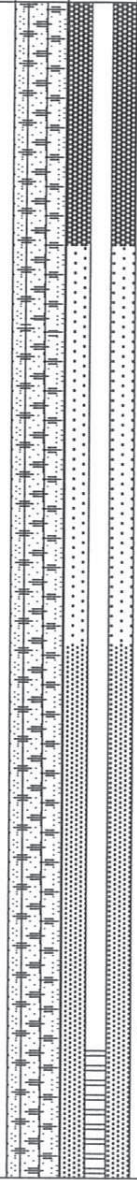
Sheet 2 of 2

Scale (ft.) (m)	Elev. (m GSD) Depth (m)	Geological Log		Strat. Plot	Monitor Details	Sample No.	Plot	Sample Type	Vapour Conc.
		Description							
55 17	332.3 18.3				Cave	7	□	RX	
60 18		End of Borehole @ 18.3 m							
65 20									
70 21									
75 23									
80 24									
85 26									
90 28									
95 29									
100 31									
105 32									
110 34									

Borehole Record

Fenco MacLaren


97/2/19

Project <u>Kenora WMMP</u> No.: <u>015926</u>		Borehole No. <u>BH-5/17</u>		
Date Drilled <u>96/1/23 - 96/1/25</u> Driller: <u>Paddock Drilling</u>		Piezometers <u>MW-5/17</u>		
Borehole Location: <u>See Figure S2-3-1</u>		Sheet 1 of 2		
Drilling Supervised by: <u>S. Bricks</u>		SS Split Spoon Sample    RX Rock Core WA Wash Sample            GR Grab Sample AU Auger Sample  Piezometer Screen CN Continuous Sample  Water Level Elev. m (with date) SH Shelby Tube Sample		
Drilling Method <u>Solid Stem Augers to 6.6 m;</u> <u>HQ Coring to 17.5 m</u>				
Piezometer Details				
Type of Pipe <u>51 mm Dia. Sch. 40 Threaded PVC</u>				
Type of Screen <u>51 mm Dia. Sch. 40 Threaded PVC, #10 Slot</u>				
Scale (ft.) (m)	Elev. (m GSD) Depth (m)	Geological Log Description	Strat. Plot Monitor Details	Sample No. PLOT Sample Type Vapour Conc.
		Ground Surface Elevation: 369.35 m		
1 5 10 15 20 25 30 35 40 45 50 16		SAND TILL - some silt to silty - with gravel and cobbles - grey-brown - dry to damp - loose to crumbly texture  - auger refusal at 6.6 m; continued with HQ coring		1 GRAB 2 SS 32 3 SS 17/15cm 4 SS 31 5 SS 25 6 SS 21 7 SS 4/10cm 8 SS 5/13cm 1 RX 2 RX 3 RX 4 RX 5 RX 6 RX 7 RX
Borehole Record	<u>S. Bricks</u> Prepared by	<u>H. Jackson</u> Checked by	<u>Fenco MacLaren</u> 97/2/19	

Project **Kenora WMMP** No.: **015926**

Borehole No. **BH-5/17**

Continuation Sheet  
Sheet 2 of 2

Scale (ft.) (m)	Elev. (m GSD) Depth (m)	Geological Log		Strat. Plot	Monitor Details	Sample No.	Plot Sample Type	Vapour Conc.
		Description						
55	352.7 16.6	probable BEDROCK - granodiorite - RQD = 63%			Cave	8	RX	
57	351.9 17.5	End of Borehole @ 17.5 m						
60								
65								
70								
75								
80								
85								
90								
95								
100								
105								
110								

Borehole Record

Fenco MacLaren

97/2/19



Project <u>Kenora WMMP</u> No.: <u>015926</u>		Borehole No. <u>BH-6/6</u>	
Date Drilled <u>96/1/25 - 96/1/25</u> Driller: <u>Paddock Drilling</u>		Piezometers <u>MW-6/5</u>	
Borehole Location: <u>See Figure S2-3-1</u>		Sheet 1 of 1	
Drilling Supervised by: <u>S. Bricks</u>		SS Split Spoon Sample    RX Rock Core WA Wash Sample            GR Grab Sample AU Auger Sample             Piezometer Screen CN Continuous Sample       Water Level Elev. SH Shelby Tube Sample      m (with date)	
Drilling Method <u>Solid Stem Augers to 5.8 m</u>			
Piezometer Details			
Type of Pipe <u>51 mm Dia. Sch. 40 Threaded PVC</u>			
Type of Screen <u>51 mm Dia. Threaded PVC, #10 Slot with Geosock</u>			

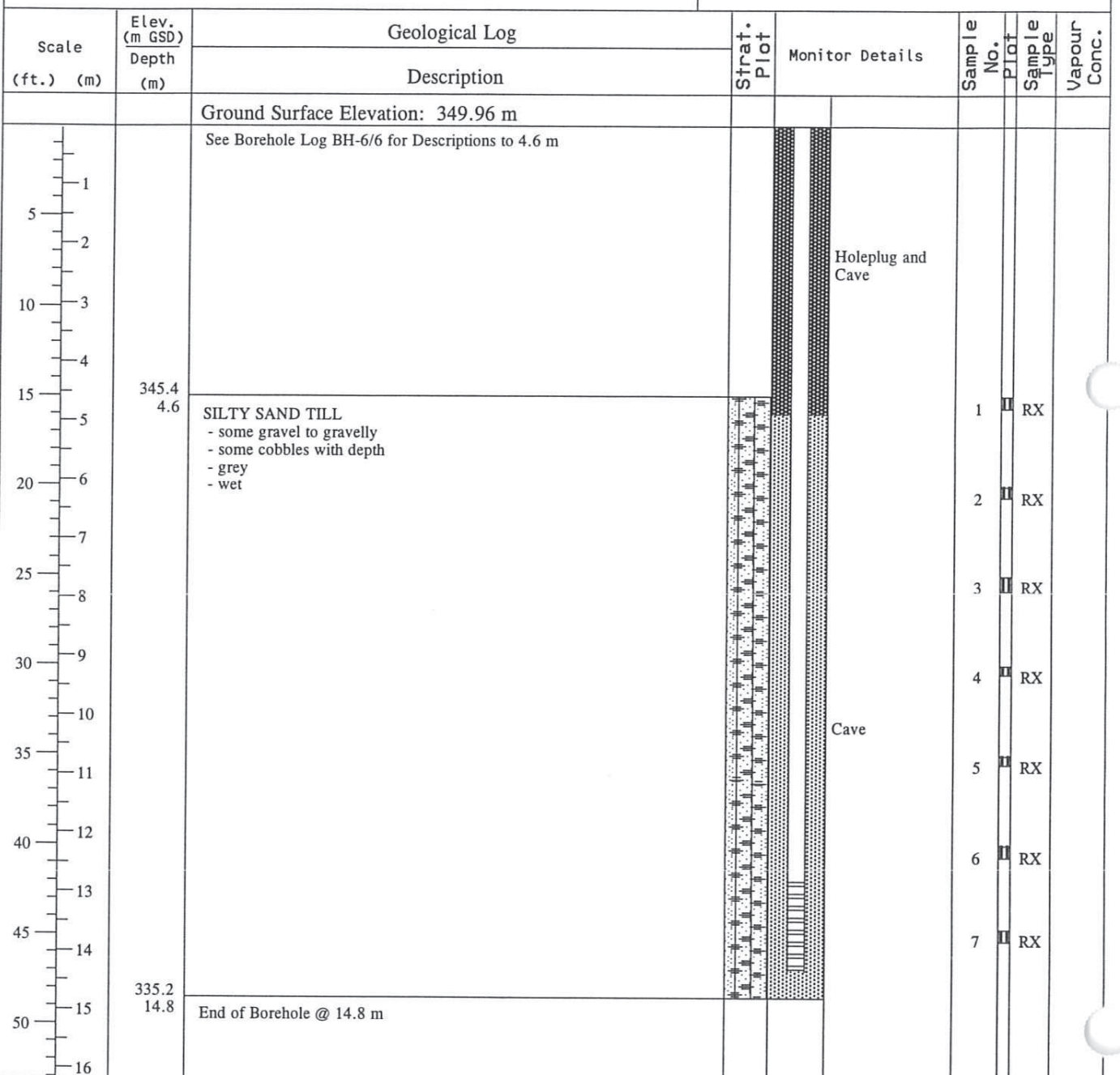
  

Scale (ft.) (m)	Elev. (m GSD) Depth (m)	Geological Log		Strat. Plot	Monitor Details	Sample No.	Plot	Sample Type	Vapour Conc.
		Description							
		Ground Surface Elevation: 350.34 m							
1 5 2 3 4 5 6 7 8 9 10 11 12 13 14 15 16	347.3 3.1  344.6 5.8	<p>SILTY SAND TILL (weathered)</p> <ul style="list-style-type: none"> <li>- sand is fine to medium grained</li> <li>- locally with thin sand or silt laminae</li> <li>- grey to brown, with rust discolouration to 1.5 m</li> <li>- root traces noted to 1.9 m</li> <li>- wet</li> <li>- loose to compact</li> </ul> <p>-----</p> <p>SILTY SAND TILL</p> <ul style="list-style-type: none"> <li>- with some thin sand laminae</li> <li>- some gravel to gravelly</li> <li>- grey</li> <li>- wet</li> </ul>			<p>Holeplug</p> <p>Cave</p> <p>Geosock on Screen</p>	1 2 3 4 5 6		SS SS SS SS SS SS	11 14 24 34 38/23cm 105
		End of Borehole @ 5.8 m							

Borehole Record	<u>S. Bricks</u> Prepared by	<u>H. Jackson</u> Checked by	<u>Fenco MacLaren</u> 97/2/19
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Project <u>Kenora WMMP</u> No.: <u>015926</u>	Borehole No. <u>BH-6/15</u>
Date Drilled <u>96/2/5 - 96/2/5</u> Driller: <u>Paddock Drilling</u>	Piezometers <u>MW-6/14</u>
Borehole Location: <u>See Figure S2-3-1</u>	Sheet 1 of 1
Drilling Supervised by: <u>S. Bricks</u>	
Drilling Method <u>Hollow Stem Augers to 4.6 m;</u> <u>HQ Coring to 14.8 m</u>	
Piezometer Details	SS Split Spoon Sample RX Rock Core WA Wash Sample GR Grab Sample AU Auger Sample  Piezometer Screen CN Continuous Sample Water Level Elev. m (with date) SH Shelby Tube Sample 
Type of Pipe <u>51 mm Dia. Sch. 40 Threaded PVC</u>	
Type of Screen <u>51 mm Dia. Sch. 40 Threaded PVC, #10 Slot</u>	



Borehole Record	<b>S. Bricks</b> Prepared by	<b>H. Jackson</b> Checked by	Fenco MacLaren 97/2/19
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Project <u>Kenora WMMP</u> No.: <u>015926</u>		Borehole No. <u>BH-7/4</u>	
Date Drilled <u>96/1/26 - 96/1/26</u> Driller: <u>Paddock Drilling</u>		Piezometers <u>MW-7/4</u>	
Borehole Location: <u>See Figure S2-3-1</u>		Sheet 1 of 1	
Drilling Supervised by: <u>S. Bricks</u>		SS Split Spoon Sample    RX Rock Core WA Wash Sample            GR Grab Sample AU Auger Sample             Piezometer Screen CN Continuous Sample      Water Level Elev. m (with date) SH Shelby Tube Sample	
Drilling Method <u>Solid Stem Augers to 4.4 m</u>			
Piezometer Details			
Type of Pipe <u>51 mm Dia. Sch. 40 Threaded PVC</u>			
Type of Screen <u>51 mm Dia. Threaded PVC, #10 Slot with Geosock</u>			

Scale (ft.) (m)	Elev. (m GSD) Depth (m)	Geological Log		Strat. Plot	Monitor Details	Sample No.	Sample Type	Vapour Conc.
		Description						
		Ground Surface Elevation: 349.47 m						
1	349.0 0.5	PEAT/ORGANICS - black				1	GRAB	
5	347.8 1.7	CLAYEY SILT - with organics - soft - wet			Holeplug	2	SS	20
10		SILTY SAND and GRAVEL (TILL-LIKE) - grey-brown - loose to compact - wet			Cave	3	SS	20
15	345.1 4.4	End of Borehole @ 4.4 m			Geosock on Screen	4	SS	17
16						5	SS	13/23cm

Borehole Record	<u>S. Bricks</u> Prepared by	<u>H. Jackson</u> Checked by	<u>Fenco MacLaren</u> 97/2/19
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Project <u>Kenora WMMP</u> No.: <u>015926</u>	Borehole No. <u>BH-8/22</u>
Date Drilled <u>96/2/14 - 96/2/15</u> Driller: <u>Paddock Drilling</u>	Piezometers <u>MW-8/22</u>
Borehole Location: <u>See Figure S2-3-1</u>	Sheet 1 of 2
Drilling Supervised by: <u>S. Bricks</u>	
Drilling Method <u>Solid Stem Augers to 13.8 m;</u> <u>HQ Coring to 21.6 m</u>	

Piezometer Details Type of Pipe <u>51 mm Dia. Sch. 40 Threaded PVC</u> Type of Screen <u>51 mm Dia. Sch. 40 Threaded PVC, #10 Slot</u>	SS Split Spoon Sample    RX Rock Core WA Wash Sample            GR Grab Sample AU Auger Sample             Piezometer Screen CN Continuous Sample       Water Level Elev. m (with date) SH Shelby Tube Sample
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Scale (ft.) (m)	Elev. (m GSD) Depth (m)	Geological Log		Strat. Plot	Monitor Details	Sample No.	Plot	Sample Type	Vapour Conc.
		Description							
		Ground Surface Elevation: 358.96 m							
		<p><b>SAND to SILTY SAND TILL</b></p> <ul style="list-style-type: none"> <li>- some gravel to gravelly</li> <li>- brown, becoming grey below 1.2 m</li> <li>- compact to very compact</li> <li>- damp to moist, wet below 10.7 m</li> <li>- with numerous thin (&lt;3 cm) sand lenses throughout</li> </ul>							
		<p>- possible thick sand and gravel lens at approximately 13.7 m</p> <p>- soils too loose to continue with solid stem augers below 13.8 m; continued with HQ coring</p>							
						1		SS	78
						2		SS	54
						3		SS	70
						4		SS	90/23cm
						5		SS	60/15cm
						6		SS	80
						7		SS	57
						8		SS	110/25cm
						9		SS	75
						10		SS	50/15cm
						11		SS	70/15cm
						12		SS	100/8cm
						13		SS	135/23cm
						14		SS	105/18cm
						15		SS	110/23cm
						1		RX	



Borehole Record	Prepared by <u>S. Bricks</u>	Checked by <u>H. Jackson</u>	Fenco MacLaren 97/2/19
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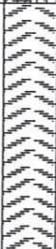


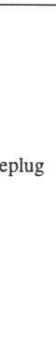

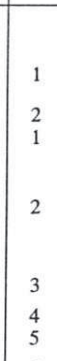




Project <b>Kenora WMMP</b> No.: <b>015926</b>	Borehole No. <b>BH-8/22</b>
Continuation Sheet Sheet 2 of 2	

Scale (ft.) (m)	Elev. (m GSD) Depth (m)	Geological Log		Strat. Plot	Monitor Details	Sample No.	Plot	Sample Type	Vapour Conc.
		Description							
55 17 60 18 19 65 20 70 21 22 75 23 80 24 25 85 26 27 90 28 29 95 30 100 31 32 105 33 110 34	337.3 21.6	End of Borehole @ 21.6 m			Cave				

Borehole Record	Fenco MacLaren  97/2/19
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Project <b>Kenora WMMP</b> No.: <b>015926</b>	Borehole No. <b>BH-9/6</b>
Date Drilled <b>96/2/15 - 96/2/15</b> Driller: <b>Paddock Drilling</b>	Piezometers <b>MW-9/6</b>
Borehole Location: <b>See Figure S2-3-1</b>	
Drilling Supervised by: <b>S. Bricks</b>	Sheet 1 of 1
Drilling Method <b>Hollow Stem Augers to 1.5 m;</b> <b>HQ Coring to 5.8 m</b>	
Piezometer Details	SS Split Spoon Sample    RX Rock Core WA Wash Sample            GR Grab Sample AU Auger Sample  Piezometer Screen CN Continuous Sample  Water Level Elev. SH Shelby Tube Sample    m (with date)
Type of Pipe <b>51 mm Dia. Sch. 40 Threaded PVC</b>	
Type of Screen <b>51 mm Dia. Sch. 40 Threaded PVC, #10 Slot</b>	

Scale (ft.) (m)	Elev. (m GSD) Depth (m)	Geological Log		Strat. Plot	Monitor Details	Sample No.	PLOT	Sample Type	Vapour Conc.	
		Description								
		Ground Surface Elevation: 352.98 m								
1 5 10 15 20 25 30 35 40 45 50 16	351.5 1.5	Primarily SILT TILL, with numerous interbeds of silt and sand - grey - dry				1		SS 18/15cm		
		BEDROCK - granodiorite - RQD = 94% - subvertical fracturing noted			Holeplug		2		SS 70/23cm	
		347.2 5.8					1		RX	
							2		RX	
							3		RX	
							4		RX	
							5		RX	
						Silica Sand		6		RX
								7		RX
			End of Borehole @ 5.8 m							

Borehole Record	<b>S. Bricks</b> Prepared by	<b>H. Jackson</b> Checked by	Fenco MacLaren 97/2/19
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Project <u>Kenora WMMP</u> No.: <u>015926</u>	Borehole No. <u>BH-10/17</u>
Date Drilled <u>96/2/16 - 96/2/16</u> Driller: <u>Paddock Drilling</u>	Piezometers <u>MW-10/17</u>
Borehole Location: <u>See Figure S2-3-1</u>	Sheet 1 of 2
Drilling Supervised by: <u>S. Bricks</u>	
Drilling Method <u>Hollow Stem Augers to 5.2 m;</u> <u>HQ Coring to 16.6 m</u>	
Piezometer Details	
Type of Pipe <u>51 mm Dia. Sch. 40 Threaded PVC</u>	SS Split Spoon Sample    RX Rock Core
Type of Screen <u>51 mm Dia. Sch. 40 Threaded PVC, #10 Slot</u>	WA Wash Sample    GR Grab Sample
	AU Auger Sample     Piezometer Screen
	CN Continuous Sample     Water Level Elev. m (with date)
	SH Shelby Tube Sample

Scale (ft.) (m)	Elev. (m GSD) Depth (m)	Geological Log Description	Strat. Plot	Monitor Details	Sample No.	PILOT	Sample Type	Vapour Conc.
		Ground Surface Elevation: 361.83 m						
		<p>SILTY SAND to SANDY SILT TILL</p> <ul style="list-style-type: none"> <li>- some gravel to gravelly, some cobbles and boulders</li> <li>- brown to grey</li> <li>- compact to dense</li> <li>- damp to moist</li> </ul> <p>- auger refusal at 5.2 m; continued with HQ coring</p>		<p>Holeplug and Cave</p> <p style="text-align: center;">Cave</p>	<p>1</p> <p>2</p> <p>3</p> <p>4</p> <p>5</p> <p>6</p> <p>1</p> <p>2</p> <p>3</p> <p>4</p> <p>5</p> <p>6</p>	<p>SS</p> <p>SS</p> <p>SS</p> <p>SS 80/23cm</p> <p>SS 90/23cm</p> <p>SS 80/15cm</p> <p>RX</p> <p>RX</p> <p>RX</p> <p>RX</p> <p>RX</p> <p>RX</p>	<p>47</p> <p>42</p> <p>85</p> <p></p> <p></p> <p></p> <p></p> <p></p> <p></p> <p></p> <p></p>	
Borehole Record	<u>S. Bricks</u> Prepared by	<u>H. Jackson</u> Checked by			Fenco MacLaren 97/2/19			

Project **Kenora WMMP** No.: **015926**

Borehole No. **BH-10/17**

Continuation Sheet

Sheet 2 of 2

Scale (ft.) (m)	Elev. (m GSD) Depth (m)	Geological Log		Strat. Plot	Monitor Details	Sample No.	PIlot	Sample Type	Vapour Conc.
		Description							
55 17 60 18 19 65 20 70 21 22 75 23 80 24 25 85 26 27 90 28 95 29 30 100 31 105 32 33 110 34	345.2 16.6	End of Borehole @ 16.6 m							

Borehole Record

Fenco MacLaren

97/2/19



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**APPENDIX N**

**Benthic Invertebrate Monitoring**

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Benthic Sampling Results - Jones Road Landfill Site												
Biological Endpoints	SB-1				SB-2				SB-R			
	Aug-00	Aug-01	Aug-02	Aug-03	Aug-00	Aug-01	Aug-02	Aug-03	Aug-00	Aug-01	Aug-02	Aug-03
Density (as number of organisms per square meter) of total benthic invertebrates	752	376	406	1540	731	969	928	4217	1612	434	507	4866
Density by taxon (as number of Families per station)	8	1	5	6	6	4	8	17	12	4	8	12



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**APPENDIX O**

**MOE Landfill Reporting Submission Forms**

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## Appendix D-Monitoring and Screening Checklist General Information and Instructions

**General Information: The checklist is to be completed, and submitted with the Monitoring Report.**

**Instructions:** A complete checklist consists of:

- (a) a completed and signed checklist, including any additional pages of information which can be attached as needed to provide further details where indicated.
- (b) completed contact information for the Competent Environmental Practitioner (CEP)
- (c) self-declaration that CEP(s) meet(s) the qualifications as set out below and in Section 1.2 of the Technical Guidance Document.

**Definition of Groundwater CEP:**

For groundwater, the CEP must have expertise in hydrogeology and meet one of the following:

- (a) the person holds a licence, limited licence or temporary licence under the *Professional Engineers Act*; or
- (b) the person holds a certificate of registration under the *Professional Geoscientists Act, 2000* and is a practicing member, temporary, member or limited member of the Association of Professional Geoscientists of Ontario. O. Reg. 66/08, s. 2..

**Definition of Surface water CEP:**

A CEP for surface water assessments is a scientist, professional engineer or professional geoscientist as described in (a) and (b) above with demonstrated experience and post-secondary education, either a diploma or degree, in hydrology, aquatic ecology, limnology, aquatic biology, physical geography with specialization in surface water, and/or water resource management.

The type of scientific work that a CEP performs must be consistent with that person's education and experience. If an individual has appropriate training and credentials in both groundwater and surface water and is responsible for both areas of expertise, the CEP may then complete and validate both sections of the checklist.

<b>Monitoring Report and Site Information</b>	
<b>Waste Disposal Site Name</b>	Kenora Area Waste Disposal Site (Jones Road Landfill)
<b>Location (e.g. street address, lot, concession)</b>	Part Lot 1 & 2, Concession II, Township of PettyPiece.
<b>GPS Location (taken within the property boundary at front gate/ front entry)</b>	412895 , 5521124
<b>Municipality</b>	District of Kenora
<b>Client and/or Site Owner</b>	City of Kenora
<b>Monitoring Period (Year)</b>	2012-2014
This Monitoring Report is being submitted under the following:	
<b>Certificate of Approval No.:</b>	A612018
<b>Director's Order No.:</b>	Type Here
<b>Provincial Officer's Order No.:</b>	Type Here
<b>Other:</b>	Type Here



Report Submission Frequency	<input type="radio"/> Annual <input checked="" type="radio"/> Other	Specify (Type Here): every three years
The site is:	<input checked="" type="radio"/> Active <input type="radio"/> Inactive <input type="radio"/> Closed	
If closed, specify C of A, control or authorizing document closure date:		Select Date
Has the nature of the operations at the site changed during this monitoring period?	<input type="radio"/> Yes <input checked="" type="radio"/> No	
If yes, provide details:	Type Here	
Have any measurements been taken since the last reporting period that indicate landfill gas volumes have exceeded the MOE limits for subsurface or adjacent buildings? (i. e. exceeded the LEL for methane)	<input type="radio"/> Yes <input checked="" type="radio"/> No	

**Groundwater WDS Verification:**

Based on all available information about the site and site knowledge, it is my opinion that:

**Sampling and Monitoring Program Status:**

<p>1) The monitoring program continues to effectively characterize site conditions and any groundwater discharges from the site. All monitoring wells are confirmed to be in good condition and are secure:</p>	<p><input checked="" type="radio"/> Yes <input type="radio"/> No</p>	<p>If no, list exceptions (Type Here):</p>
<p>2) All groundwater, leachate and WDS gas sampling and monitoring for the monitoring period being reported on was successfully completed as required by Certificate(s) of Approval or other relevant authorizing/control document(s):</p>	<p><input type="radio"/> Yes <input checked="" type="radio"/> No <input type="radio"/> Not Applicable</p>	<p>If no, list exceptions below or attach information.</p>

Groundwater Sampling Location	Description/Explanation for change (change in name or location, additions, deletions)	Date
MW4/6	damaged in 2012 and 2013, no samples collected until August 2013, when monitor was replaced	Select Date
MW5/17	Dry in 2012, 2013 and May 2014, no samples collected	Select Date
MW11/4	Dry in May 2012 and both events in 2013. No samples collected	Select Date
MW24/5	sample tubing damaged in in August 2013 and May 2014, no samples collected. repaired and sample collected in August 2014	Select Date

<b>3) a) Some or all groundwater, leachate and WDS gas sampling and monitoring requirements have been established or defined outside of a ministry C of A, authorizing, or control document.</b>		<input type="radio"/> Yes <input checked="" type="radio"/> No <input type="radio"/> Not Applicable
<b>b) If yes, the sampling and monitoring identified under 3(a) for the monitoring period being reported on was successfully completed in accordance with established protocols, frequencies, locations, and parameters developed as per the Technical Guidance Document:</b>		<input type="radio"/> Yes <input type="radio"/> No <input type="radio"/> Not Applicable
		If no, list exceptions below or attach additional information.
<b>Groundwater Sampling Location</b>	<b>Description/Explanation for change (change in name or location, additions, deletions)</b>	<b>Date</b>
Type Here	Type Here	Select Date
Type Here	Type Here	Select Date
Type Here	Type Here	Select Date
Type Here	Type Here	Select Date
<b>4) All field work for groundwater investigations was done in accordance with standard operating procedures as established/outlined per the Technical Guidance Document (including internal/external QA/QC requirements) (Note: A SOP can be from a published source, developed internally by the site owner's consultant, or adopted by the consultant from another organization):</b>		<input checked="" type="radio"/> Yes <input type="radio"/> No  If no, specify (Type Here):

## Sampling and Monitoring Program Results/WDS Conditions and Assessment:

<p>5) The site has an adequate buffer, Contaminant Attenuation Zone (CAZ) and/or contingency plan in place. Design and operational measures, including the size and configuration of any CAZ, are adequate to prevent potential human health impacts and impairment of the environment.</p>	<p><input checked="" type="radio"/> Yes <input type="radio"/> No</p>	<p>If no, the potential design and operational concerns/exceptions are as follows (Type Here):</p>	
<p>6) The site meets compliance and assessment criteria.</p>	<p><input checked="" type="radio"/> Yes <input type="radio"/> No</p>	<p>If no, list and explain exceptions (Type Here):</p>	
<p>7) The site continues to perform as anticipated. There have been no unusual trends/ changes in measured leachate and groundwater levels or concentrations.</p>	<p><input checked="" type="radio"/> Yes <input type="radio"/> No</p>	<p>If no, list exceptions and explain reason for increase/change (Type Here):</p>	
<p>1) Is one or more of the following risk reduction practices in place at the site:</p> <p>(a) There is minimal reliance on natural attenuation of leachate due to the presence of an effective waste liner and active leachate collection/treatment; or</p> <p>(b) There is a predictive monitoring program in-place (modeled indicator concentrations projected over time for key locations); or</p> <p>(c) The site meets the following two conditions (typically achieved after 15 years or longer of site operation):</p> <p><i>i.</i>The site has developed stable leachate mound(s) and stable leachate plume geometry/concentrations; and</p> <p><i>ii.</i>Seasonal and annual water levels and water quality fluctuations are well understood.</p>	<p><input checked="" type="radio"/> Yes <input type="radio"/> No</p>	<p>Note which practice(s):</p>	<p><input type="checkbox"/> (a) <input checked="" type="checkbox"/> (b) <input checked="" type="checkbox"/> (c)</p>
<p>9) Have trigger values for contingency plans or site remedial actions been exceeded (where they exist):</p>	<p><input type="radio"/> Yes <input checked="" type="radio"/> No <input type="radio"/> Not Applicable</p>	<p>If yes, list value(s) that are/have been exceeded and follow-up action taken (Type Here):</p>	

## Groundwater CEP Declaration:

I am a licensed professional Engineer or a registered professional geoscientist in Ontario with expertise in hydrogeology, as defined in Appendix D under Instructions. Where additional expertise was needed to evaluate the site monitoring data, I have relied on individuals who I believe to be experts in the relevant discipline, who have co-signed the compliance monitoring report or monitoring program status report, and who have provided evidence to me of their credentials.

I have examined the applicable Certificate of Approval and any other environmental authorizing or control documents that apply to the site. I have read and followed the Monitoring and Reporting for Waste Disposal Sites Groundwater and Surface Water Technical Guidance Document (MOE, 2010, or as amended), and associated monitoring and sampling guidance documents, as amended from time to time. I have reviewed all of the data collected for the above-referenced site for the monitoring period(s) identified in this checklist. Except as otherwise agreed with the ministry for certain parameters, all of the analytical work has been undertaken by a laboratory which is accredited for the parameters analysed to *ISO/IEC 17025:2005 (E)- General requirements for the competence of testing and calibration laboratories*, or as amended from time to time by the ministry.


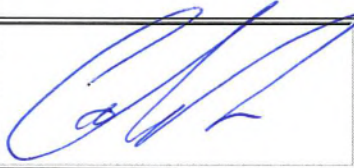
If any exceptions or potential concerns have been noted in the questions in the checklist attached to this declaration, it is my opinion that these exceptions and concerns are minor in nature and will be rectified for the next monitoring/reporting period. Where this is not the case, the circumstances concerning the exception or potential concern and my client's proposed action have been documented in writing to the Ministry of the Environment District Manager in a letter from me dated:

11-Jun-2015

## Recommendations:

Based on my technical review of the monitoring results for the waste disposal site:

<p><input checked="" type="radio"/> No changes to the monitoring program are recommended</p> <p><input type="radio"/> The following change(s) to the monitoring program is/are recommended:</p>	<p>Type Here</p>
<p><input checked="" type="radio"/> No Changes to site design and operation are recommended</p> <p><input type="radio"/> The following change(s) to the site design and operation is/are recommended:</p>	<p>Type Here</p>

<b>Name:</b>	Colin Ross		
<b>Seal:</b>	Add Image 		
<b>Signature:</b>		<b>Date:</b>	Select Date <i>June 19/15</i>
<b>CEP Contact Information:</b>	Colin Ross		
<b>Company:</b>	Azimuth Environmental Consulting Inc.		
<b>Address:</b>	85 Bayfield Street, Barrie, ON L4M 3A7		
<b>Telephone No.:</b>	705-721-8451	<b>Fax No. :</b>	705-5721-8926
<b>E-mail Address:</b>	colin@azimuthenvironmental.com		
<b>Co-signers for additional expertise provided:</b>			
<b>Signature:</b>		<b>Date:</b>	Select Date
<b>Signature:</b>		<b>Date:</b>	Select Date

## Surface Water WDS Verification:

Provide the name of surface water body/bodies potentially receiving the WDS effluent and the approximate distance to the waterbody (including the nearest surface water body/bodies to the site):

Name (s)	Morgan Lake
Distance(s)	1.9 km

Based on all available information and site knowledge, it is my opinion that:

### Sampling and Monitoring Program Status:

1) The current surface water monitoring program continues to effectively characterize the surface water conditions, and includes data that relates upstream/background and downstream receiving water conditions:	<input checked="" type="radio"/> Yes <input type="radio"/> No	If no, identify issues (Type Here):
2) All surface water sampling for the monitoring period being reported was successfully completed in accordance with the Certificate(s) of Approval or relevant authorizing/control document(s) (if applicable):	<input checked="" type="radio"/> Yes <input type="radio"/> No <input type="radio"/> Not applicable (No C of A, authorizing / control document applies)	If no, specify below or provide details in an attachment.

Surface Water Sampling Location	Description/Explanation for change (change in name or location, additions, deletions)	Date
SW-3	location dry in August 2012, 2013, 2014, no samples collected	Select Date
Type Here	Type Here	Select Date
Type Here	Type Here	Select Date
Type Here	Type Here	Select Date

<p>3) a) Some or all surface water sampling and monitoring program requirements for the monitoring period have been established outside of a ministry C of A or authorizing/control document.</p>	<p> <input type="radio"/> Yes  <input checked="" type="radio"/> No  <input type="radio"/> Not Applicable         </p>
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<p>b) If yes, all surface water sampling and monitoring identified under 3 (a) was successfully completed in accordance with the established program from the site, including sampling protocols, frequencies, locations and parameters) as developed per the Technical Guidance Document:</p>	<p> <input type="radio"/> Yes  <input type="radio"/> No  <input type="radio"/> Not Applicable         </p>	<p>If no, specify below or provide details in an attachment.</p>
--	--	--

Surface Water Sampling Location	Description/Explanation for change (change in name or location, additions, deletions)	Date
Type Here	Type Here	Select Date
Type Here	Type Here	Select Date
Type Here	Type Here	Select Date
Type Here	Type Here	Select Date

<p>4) All field work for surface water investigations was done in accordance with standard operating procedures, including internal/external QA/QC requirements, as established/ outlined as per the Technical Guidance Document, MOE 2010, or as amended. (Note: A SOP can be from a published source, developed internally by the site owner's consultant, or adopted by the consultant from another organization):</p>	<p> <input checked="" type="radio"/> Yes  <input type="radio"/> No         </p>	<p>If no, specify (Type Here):</p>
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## Sampling and Monitoring Program Results/WDS Conditions and Assessment:

<b>5) The receiving water body meets surface water-related compliance criteria and assessment criteria: i.e., there are no exceedances of criteria, based on MOE legislation, regulations, Water Management Policies, Guidelines and Provincial Water Quality Objectives and other assessment criteria (e.g., CWQGs, APVs), as noted in Table A or Table B in the Technical Guidance Document (Section 4.6):</b>	<input checked="" type="radio"/> <b>Yes</b> <input type="radio"/> <b>No</b>
--	--

**If no, list parameters that exceed criteria outlined above and the amount/percentage of the exceedance as per the table below or provide details in an attachment:**

Parameter	Compliance or Assessment Criteria or Background	Amount by which Compliance or Assessment Criteria or Background Exceeded
e.g. Nickel	e.g. C of A limit, PWQO, background	e.g. X% above PWQO
Total Phosphorus	0.03 mg/L	maximum exceedance for period of record (2012-2014) is 0.31 mg/L or 1033%
Iron	0.3 mg/L	maximum exceedance for period of record (2012-2014) is 3.8 mg/L or 1266%
phenols	0.001	maximum exceedance for period of record (2012-2014) is 0.009 mg/L or 900%
Type Here	Type Here	Type Here

<b>6) In my opinion, any exceedances listed in Question 5 are the result of non-WDS related influences (such as background, road salting, sampling site conditions)?</b>	<input checked="" type="radio"/> <b>Yes</b> <input type="radio"/> <b>No</b>	<p>All these concentrations are within the range observed at background locations (SW-3 &amp; 4) indicating they are natural in source. The total phosphorus and phenols concentrations are typical of stagnant wetland type conditions, while elevated iron concentrations are also observed in background or non-impacted ground water monitoring wells supporting this as a naturally occurring earth element source.</p>
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<p>7) All monitoring program surface water parameter concentrations fall within a stable or decreasing trend. The site is not characterized by historical ranges of concentrations above assessment and compliance criteria.</p>	<p><input type="radio"/> Yes</p> <p><input checked="" type="radio"/> No</p>	<p>Some increasing trends are noted at SW-1 and to a lesser extent at SW-2 for leachate indicator parameters. This trend is just beginning to develop and is not surprising given the age of the landfill site. These trends will be tracked as the monitoring program progresses.</p>
<p>8) For the monitoring program parameters, does the water quality in the groundwater zones adjacent to surface water receivers exceed assessment or compliance criteria (e.g., PWQOs, CWQGs, or toxicity values for aquatic biota (APVs)):</p>	<p><input type="radio"/> Yes</p> <p><input checked="" type="radio"/> No</p> <p><input type="radio"/> Not Known</p> <p><input type="radio"/> Not Applicable</p>	
<p>9) Have trigger values for contingency plans or site remedial actions been exceeded (where they exist):</p>	<p><input type="radio"/> Yes</p> <p><input checked="" type="radio"/> No</p> <p><input type="radio"/> Not Applicable</p>	<p>If yes, list value(s) that are/have been exceeded and follow-up action taken (Type Here)</p>

## Surface Water CEP Declaration:

I, the undersigned hereby declare that I am a Competent Environmental Practitioner as defined in Appendix D under Instructions, holding the necessary level of experience and education to design surface water monitoring and sampling programs, conduct appropriate surface water investigations and interpret the related data as it pertains to the site for this monitoring period.

I have examined the applicable Certificate of Approval and any other environmental authorizing or control documents that apply to the site. I have read and followed the Monitoring and Reporting for Waste Disposal Sites Groundwater and Surface Water Technical Guidance Document (MOE, 2010, or as amended) and associated monitoring and sampling guidance documents, as amended from time to time. I have reviewed all of the data collected for the above-referenced site for the monitoring period(s) identified in this checklist. Except as otherwise agreed with the ministry for certain parameters, all of the analytical work has been undertaken by a laboratory which is accredited for the parameters analysed to *ISO/IEC 17025:2005 (E)- General requirements for the competence of testing and calibration laboratories*, or as amended from time to time by the ministry.

If any exceptions or potential concerns have been noted in the questions in the checklist attached to this declaration, it is my opinion that these exceptions and concerns are minor in nature or will be rectified for future monitoring events. Where this is not the case, the circumstances concerning the exception or potential concern and my client's proposed action have been documented in writing to the Ministry of the Environment District Manager in a letter from me dated:

11-Jun-2015

## Recommendations:

Based on my technical review of the monitoring results for the waste disposal site:

<p><input checked="" type="radio"/> No Changes to the monitoring program are recommended</p> <p><input type="radio"/> The following change(s) to the monitoring program is/are recommended:</p>	<p>Type Here</p>
<p><input checked="" type="radio"/> No changes to the site design and operation are recommended</p> <p><input type="radio"/> The following change(s) to the site design and operation is/are recommended:</p>	<p>Type Here</p>

<b>CEP Signature</b>		
<b>Relevant Discipline</b>	Water Quality Assessor	
<b>Date:</b>	11-Jun-2015	
<b>CEP Contact Information:</b>	Colin Ross	
<b>Company:</b>	Azimuth Environmental Consulting Inc.	
<b>Address:</b>	85 Bayfield Street, Suite 400, Barrie, ON. L4M 3A7	
<b>Telephone No.:</b>	705-721-8451	
<b>Fax No. :</b>	705-721-8926	
<b>E-mail Address:</b>	colin@azimuthenvironmental.com	
<b>Save As</b>		<b>Print Form</b>